

PO Box 220 19 King Edward Street Ulverstone Tasmania 7315 Tel (03) 6429 8900

admin@centralcoast.tas.gov.au www.centralcoast.tas.gov.au

<mark>Carolyn Harris</mark>

Application for Planning

S.57 Land Use Planning and Approvals Act 1993

The following application has been received:

Application No.:	DA2023255
Location:	Harveys Road (CT131560/2), North Motton
Proposal:	Residential - single dwelling
Performance Criteria:	Discretionary uses; and access for new dwellings

The application may be inspected at the Administration Centre, 19 King Edward Street, Ulverstone during Office hours and on the council's website: www.centralcoast.tas.gov.au. Any person may make representation in relation to the applications (in accordance with S.57(5) of the Act) by writing to the General Manager, PO Box 220, Ulverstone 7315 or by email to admin@centralcoast.tas.gov.au and quoting the Application No. Any representations received by the Council are classed as public documents and will be made available to the public where applicable under the Local Government (Meeting Procedures) Regulations 2015.

The representation must be made on or before

7 May 2024

Date of Notification: 20 April 2024

CENTRAL COAST COUNCIL
PO Box 220
19 King Edward Street
ULVERSTONE TASMANIA 7315
Ph: (03) 6429 8900
Email: planning@centralcoast.tas.gov.au
www: centralcoast.tas.gov.au

C	CENTRAL COAST COUNCIL	
CENTRAL COAST	CENTRAL COAST COUNCIL LAND USE PLANNING	

Received:

Doc ID:

Application No:

18/04/2024

DA2023255

483361

Land Use Planning and Approvals Act 1993

Tasmanian Planning Scheme – Central Coast

PLANNING PERMIT APPLICATION

Zone:

Office use only:

Permit Pathway – NPR/Permitted/Discretionary

Use or Development Site:									
Site Address		Lot 2 Harveys Rd, North Motton TAS 7315							
Certificate of Title Reference			131560/2						
Land Area		187934 sqm	Heritage Li	sted Pro	perty	NC	∇	YES	
Applicant(s)									
First Name(s)	Cł	nloe		Sur	name(s)	Overton			
Company name (if applicable)	Ec	clo Designs Contact No: 0419387746							
Postal Address:	4	4 Riverbend Drive Don TAS 7310							
Email address:	ad	min@eclodesign	s.com						
Please tick box to receiv	vive correspondence and any relevant information regarding your application via email.								
Owner(s) (note	– if n	nore than one owne	r, all names must be in	dicated)					
First Name(s)	Ма	rk & Kim		Middl	e Names	(s)			
Surname(s)	Phi	lips-Haines		Comp	bany nam	le (if applica	ble)]
	4.17	Volkor Street L	veretene TAC 70	15]
Postal Address:	1 Walker Street Ulverstone TAS 7315								

PERMIT APP	LICATION INFORMATION	(If insufficient space for proposed use and development, please attach separate documents)
"USE" is the purp	ose or manner for which land is utilised.	
Proposed Use	Dwelling	
Use Class Office use only		
buildings and stru Proposed De	actures, signs, any change in ground level an	entation in PDF format to planning@centralcoast.tas.gov.au

Value of the development – (to include all works on site such as outbuildings, sealed driveways and fencing)
\$. 4 . 5 . 0 . 0 . 0
Total floor area of the development

Declaration of Notice to Landowner			
If land is NOT in the applic	cant's ownership		
-	, declare that the owner/each of the owners of the land has a the land has a sit application under section 52(1) of the Land Use Planning and		
Signature of Applicant	llee Date 2	20/9/2023	
If the application involves	s land within a Strata Corporation		
	declare that the owner/each of the owners of the body corponate this permit application.	oration has been	
Signature of Applicant	Date		

If the application involves land owned or administered by the CENTRAL COAST COUNCIL					
Central Coast Council co	Central Coast Council consents to the making of this permit application.				
General Managers Signa	ature	Date			
If the permit application involves land owned or administered by the CROWN					
I, Jesse Walker, the Minister's delegate responsible for the land, consent to the					
making of this permit application.					
Delegate (Signature)	Date 12/	24/2024			

NB: If the site includes land owned or administered by the Central Coast Council or by a State government agency, the consent in writing (a letter) from the Council or the Minister responsible for Crown land must be provided at the time of making the application - and this application form must be signed by the Council or the Minister responsible.

Applicants Declaration

Signature of Applicant/s

I/ we <u>Chloe Overton</u> declare that the information I have given in this permit application to be true and correct to the best of my knowledge.

00

____ Date <u>20/9/2023</u>___

Office Use Only	
Planning Permit Fee	\$
Public Notice Fee	\$
Permit Amendment / Extension Fee	\$
No Permit Required Assessment Fee	\$
TOTAL	\$
Validity Date	

Notice of Termination of Authority and Instrument of Delegation

DELEGATION OF THE DIRECTOR-GENERAL OF LANDS' FUNCTIONS UNDER THE LAND USE PLANNING AND APPROVALS ACT 1993

I, JASON JACOBI, being and as the Director-General of Lands appointed under section 7 of the *Crown Lands Act 1976*, acting pursuant to section 23AA(5A) of the *Acts Interpretation Act 1931*, hereby give notice that the authority of the holders of the offices of Deputy Secretary (Parks and Wildlife Service) (position number 700451), General Manager (Park Operations and Business Services) (position number 708581), Director (Operations) (position number 708050), Manager (Property Services) (position number 707556), Unit Manager (Operations) (position number 702124) and Team Leader (Assessments) (position number 334958) to perform the functions conferred on the Director-General of Lands, as delegated on 13 December 2022 by Michael Pervan, then Director-General of Lands, is terminated with immediate effect.

Further, acting pursuant to section 52(1E) of the Land Use Planning and Approvals Act 1993 ("the Act"), I hereby delegate the functions described (by reference to the relevant provision of the Act and generally) in Schedule I, to the persons respectively holding the offices of Deputy Secretary (Parks and Wildlife Service) (position number 700451), General Manager (Park Operations and Business Services) (position number 708581), Manager (Property Services) (position number 707556), Unit Manager (Operations) (position number 702124) and Team Leader (Assessments) (position number 334958) in accordance with the functions delegated to me by the Minister for Parks, being and as the Minister administering the *Crown Lands Act 1976*, by instrument dated 9 November 2023.

SCHEDULE I

Provision

Description of Functions

SectionSigning, and providing written permission for, applications for
permits in relation to Crown land.

Dated at HOBART this 28th day of November 2023



Jason Jacobi DIRECTOR-GENERAL OF LANDS



Department of Natural Resources, and Environment Tasmania



GPO Box 44, Hobart, TAS 7001 Australia Ph 1300 TAS PARKS / 1300 827 727 Fax 03) 6223 8308 www.parks.tas.gov.au

> Enquiries: Tanya Simm Phone: 6165 4691 Email: Tanya.Simm@parks.tas.gov.au Our ref: 23/7776

> > CENTRAL COAST COUNCIL LAND USE PLANNING

> > > 16/04/2024

DA2023255

483176

12 April 2024

Ms Chloe Overton Eclo Designs 4 Riverbend Drive DON TAS 7310

E: admin@eclodesigns.com

Dear Ms Overton,

LODGEMENT OF PLANNING APPLICATION CHLOE OVERTON, ECLO DESIGNS DWELLING HARVEYS RD, NORTH MOTTON

Received:

Doc ID:

Application No:

This letter, issued pursuant to section 52(1B) of the *Land Use Planning and Approvals Act 1993* (LUPAA), is to confirm that the Crown consents to the making of the enclosed Planning Permit Application, insofar as the proposed development relates to Crown land managed by the Department of Natural Resources and Environment Tasmania.

Crown consent is only given to the lodgement of this application. Any variation will require further consent from the Crown.

It is Departmental policy that all fire buffer areas (Hazard Management Areas and Fuel Modified Areas) are maintained wholly within freehold title boundaries and not on neighbouring Crown or Reserved land. Additionally, it is not the Parks and Wildlife Service's practice for the Crown to enter into agreements under Part 5 of LUPAA in support of developments on private property.

The Department will continue to assess the application to licence the subject Crown land.

This letter does not constitute, nor imply, any approval to undertake works, or that any other approvals required under the *Crown Lands Act 1976* have been granted. If planning approval is given for the proposed development, the applicant will be required to obtain separate and distinct consent from the Crown before commencing any works on Crown land.

If you need more information regarding the above, please contact the officer nominated at the head of this correspondence.

Yours sincerely,

Jesse Walker Team Leader (Assessments)



Received:

Doc ID:

Application No:

CENTRAL COAST COUNCIL LAND USE PLANNING

DA2023255

465673



SEARCH OF TORRENS TITLE

VOLUME	FOLIO
131560	2
EDITION	DATE OF ISSUE
8	11-Mar-2020

SEARCH DATE : 11-Sep-2023 SEARCH TIME : 03.58 PM

DESCRIPTION OF LAND

Parish of NORTH MOTTON, Land District of DEVON Lot 2 on Plan 131560 Derivation : Part of Lot 17157,183A-2R-0P & Part of Lot 17116, 22A-2R-27P gtd to George Lewis Elmer Purton. Prior CTs 120728/1 and 48356/4

SCHEDULE 1

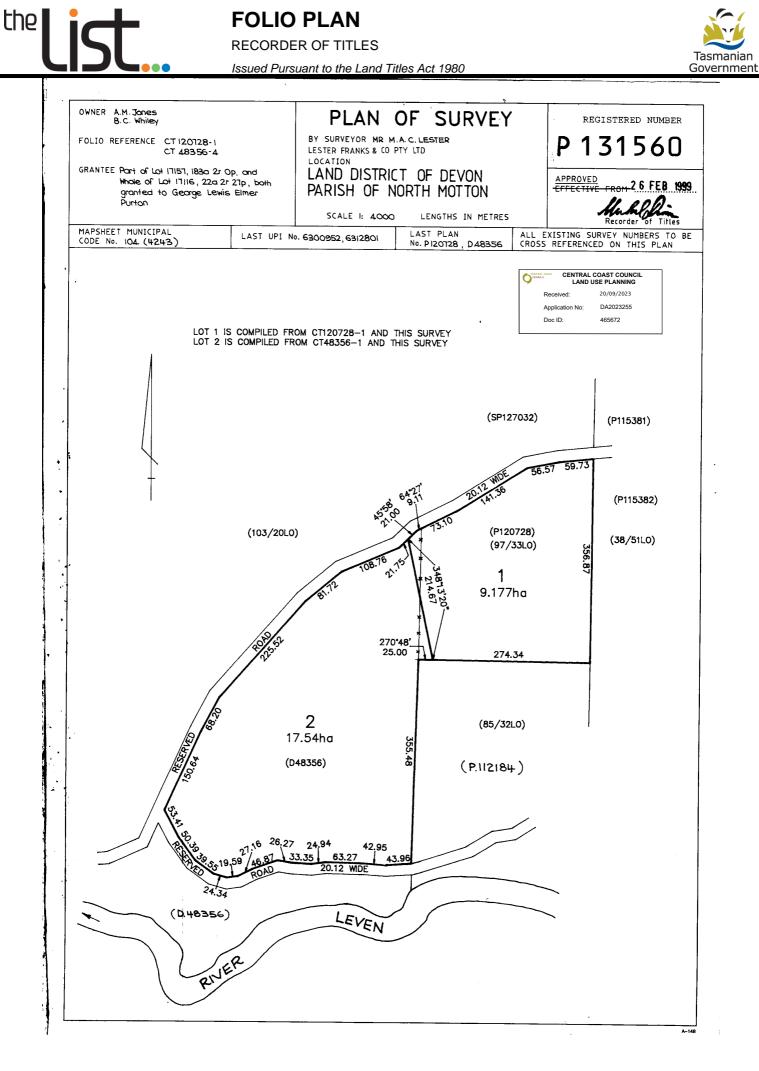
M650703 & E210919 MARK ANTHONY PHILLIPS-HAINES and KIM LOUISE PHILLIPS-HAINES as tenants in common in equal shares Registered 11-Mar-2020 at noon

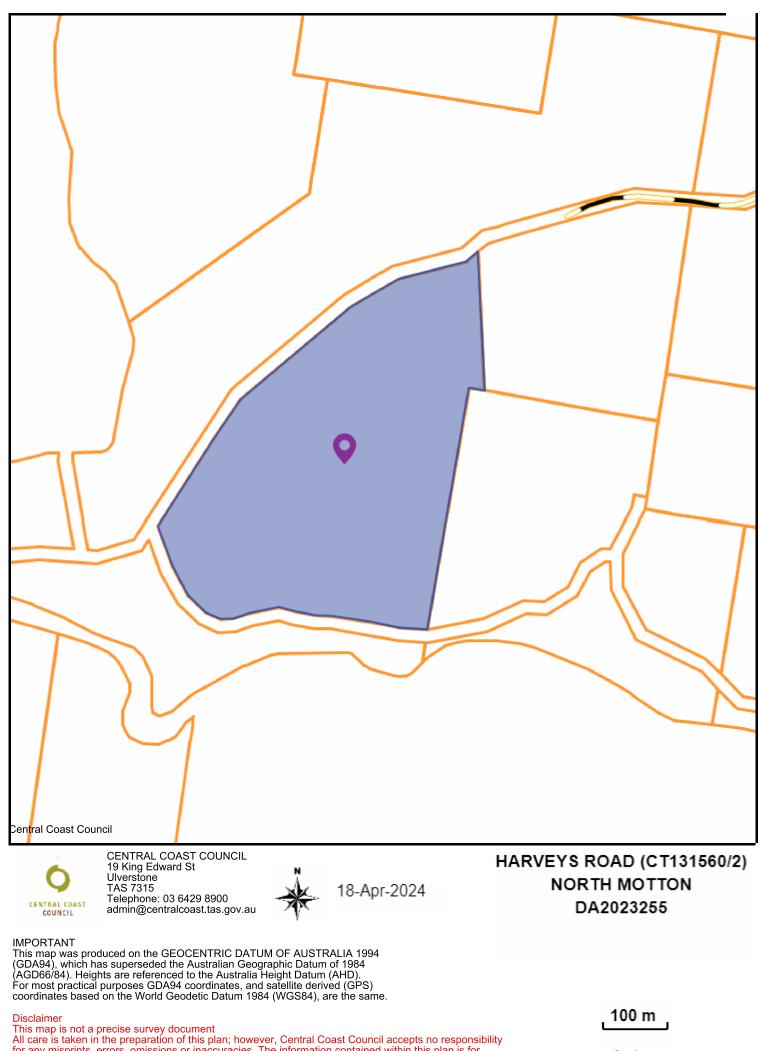
SCHEDULE 2

Reservations and conditions in the Crown Grant if any

UNREGISTERED DEALINGS AND NOTATIONS

No unregistered dealings or other notations

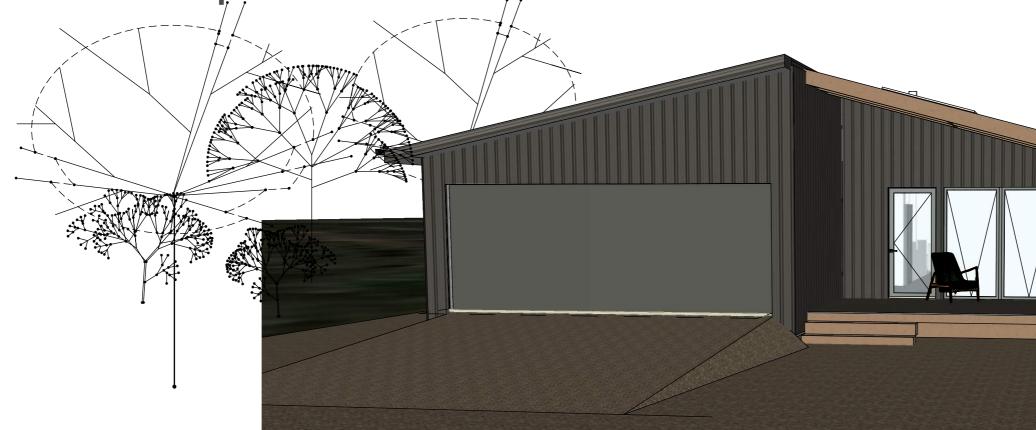




All care is taken in the preparation of this plan; however, Central Coast Council accepts no responsibility for any misprints, errors, omissions or inaccuracies. The information contained within this plan is for pictorial representation only. Do not scale. Accurate measurement should be undertaken by survey. © The List 2023. © Central Coast Council 2023.

Scale = 1:5881.680

Proposed Residence AT Harveys Road, North Motton, 7315 FOR Mark & Kim Phillips-Haines



SITE INFORMATION

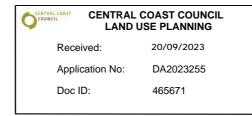
LAND TITLE REFERENCE: **131560/2** WIND CLASSIFICATION: **N3** SOIL CLASSFICATION: **P/A** CLIMATE ZONE: **7** BAL LEVEL: **TBC** TBC ALPINE OR SUB-ALPINE AREA: **N/A** CORROSION ENVIRONMENT: **N/A** OTHER HAZARDS: **MEDIUM LANDSLIP** PID: **1906437** ZONING: **RURAL**

AREA SCHEDULE

SITE AREA: 187934sqm

GROUND FLOOR AREA: 198.9m2/21.4 SQ.

DECK AREA: 33.3m2



SITE PLAN SITE PLAN ENLARGED SITE PLAN ISOMETRIC VIEWS ISOMETRIC VIEWS FLOOR PLAN ELEVATION 1 & 2 ELEVATION 3 & 4 SETOUT PLAN ROOF PLAN INTERNAL PLUMBING PLAN ELECTRICAL PLAN WINDOW & DOOR SCHEDULE WINDOW & DOOR NOTES ACCESS PLAN





CONTOURS AT 10m INTERVALS

SITE PLAN LEGEND & NOTES:

GENERAL NOTES:

DURING CONSTRUCTION SOIL AND WATER IS TO BE APPROPRIATELY MANAGED. THIS INCLUDES THE PROVISION OF SILT FENCING, FILTER SCREENS OR DEDICATED SILT TRAPS TO PREVENT DISCHARDGE OF GRAVEL, SOIL OR OTHER DEBRIS TO ANY EXISTING WATER COURSE OR ADJOINING PROPERTY DURING THE CONSTRUCTION PROCESS.

EXCAVATION:

ALLOW FOR BULK EXCAVATION WHERE REQUIRED AND ALL EXCAVATION, FILLING, BACK FILLING AND CONSOLIDATION REQUIRED FOR THE FOOTINGS AND SLAB, RETAIN ALL ACCESS AND SERVICES INDICATED. MAKE GOOD.

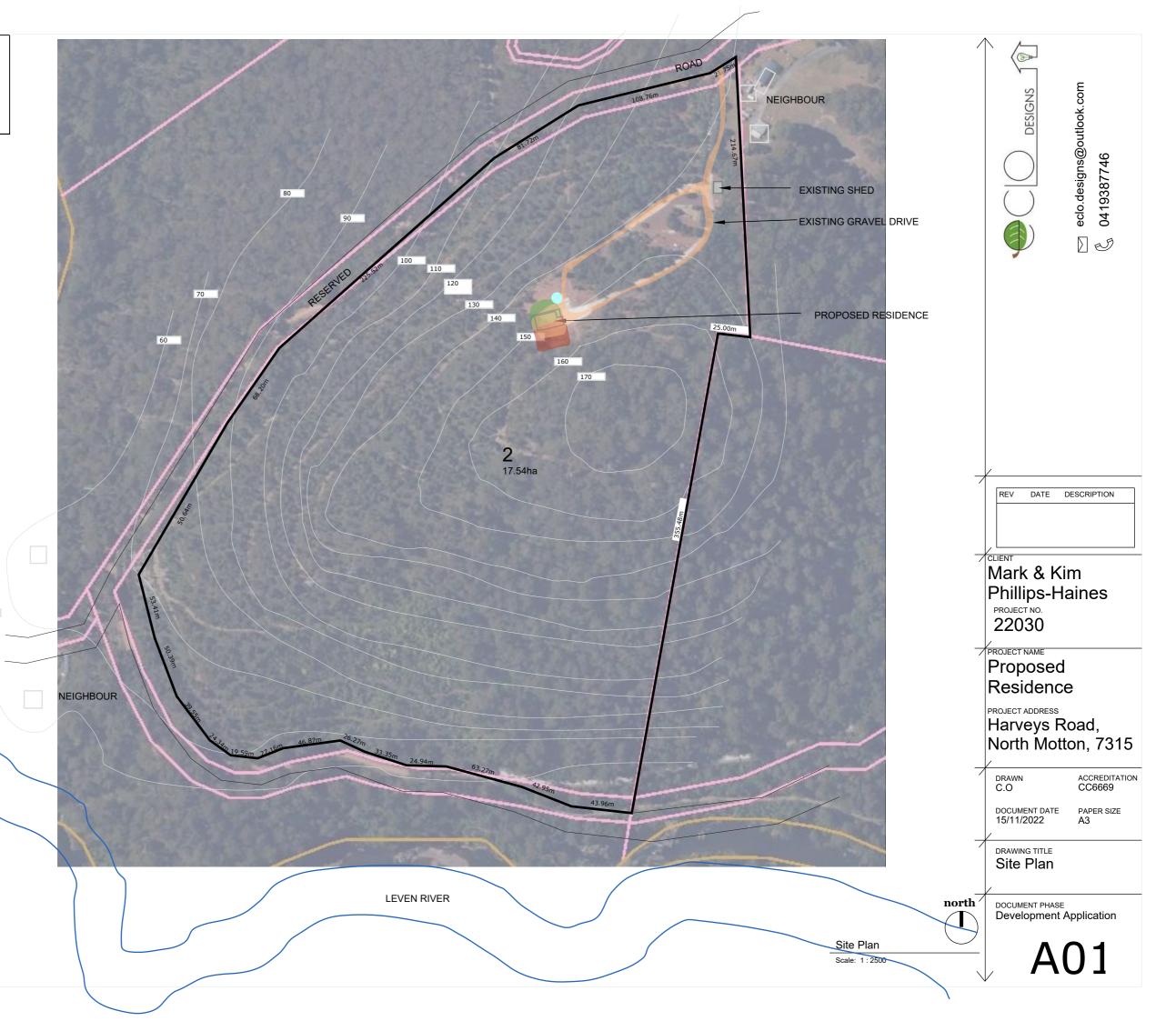
SETTING OUT:

THE CLIENT IS RESPONSIBLE FOR VERIFYING THE BOUNDARY PEGS ARE IN THE CORRECT LOCATION, MARKED AND CLEARLY VISIBLE FOR THE BUILDER. THE BUILDER SHALL ACCURATELY SET-OUT THE WORKS AND VERIFY ALL DIMENSIONS AND LEVELS BEFORE COMMENCING ANY WORKS. AND SHALL MAKE GOOD AT HIS OWN EXPENSE ANY ERRORS ARISING FROM INACCURACIES OF THE SETOUT.

PROTECTION WORK:

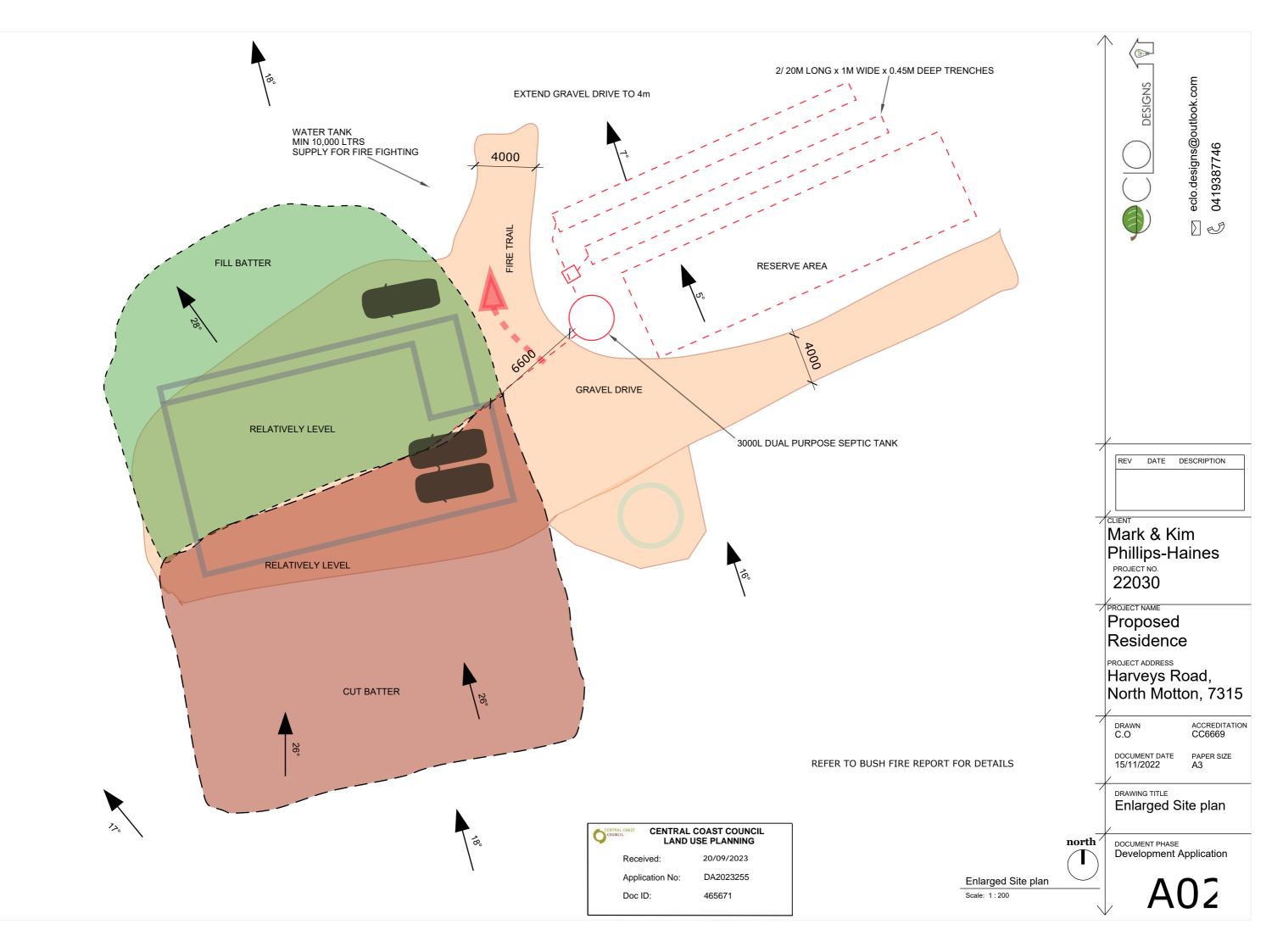
(SECTION 121 OF THE BUILDING ACT) IF EXCAVATION IS TO A LEVEL BELOW THAT OF THE ADJOINING OWNER'S FOOTINGS, ALONG THE TITLE BOUNDARY OR WITHIN 3 METRES OF A BUILDING BELONGING TO AN ADJOINING OWNER, THE BUILDER MUST (AS A MINIUMUM) PROVIDE AND MAINTAIN A GUARD TO SUPERVISE THE EXCAVATION. ADJOINING OWNER TO BE NOTIFIED USING FORM 6 (BUILDING AND PROTECTION WORK NOTICE) BY THE BUILDING SURVEYOR.

50



SITE SERVICES:

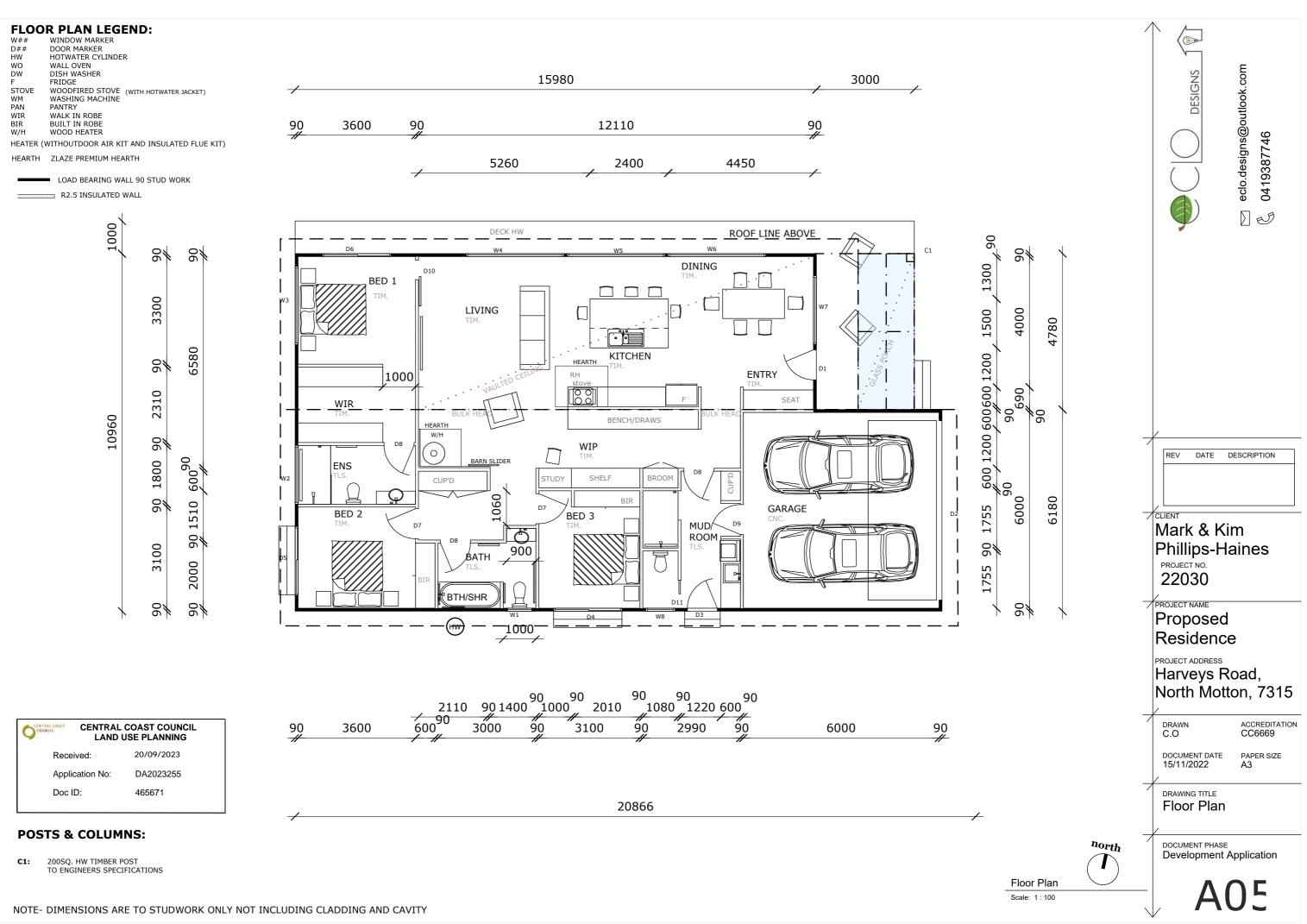
NO SERVICES AVALIABLE FOR THE SITE

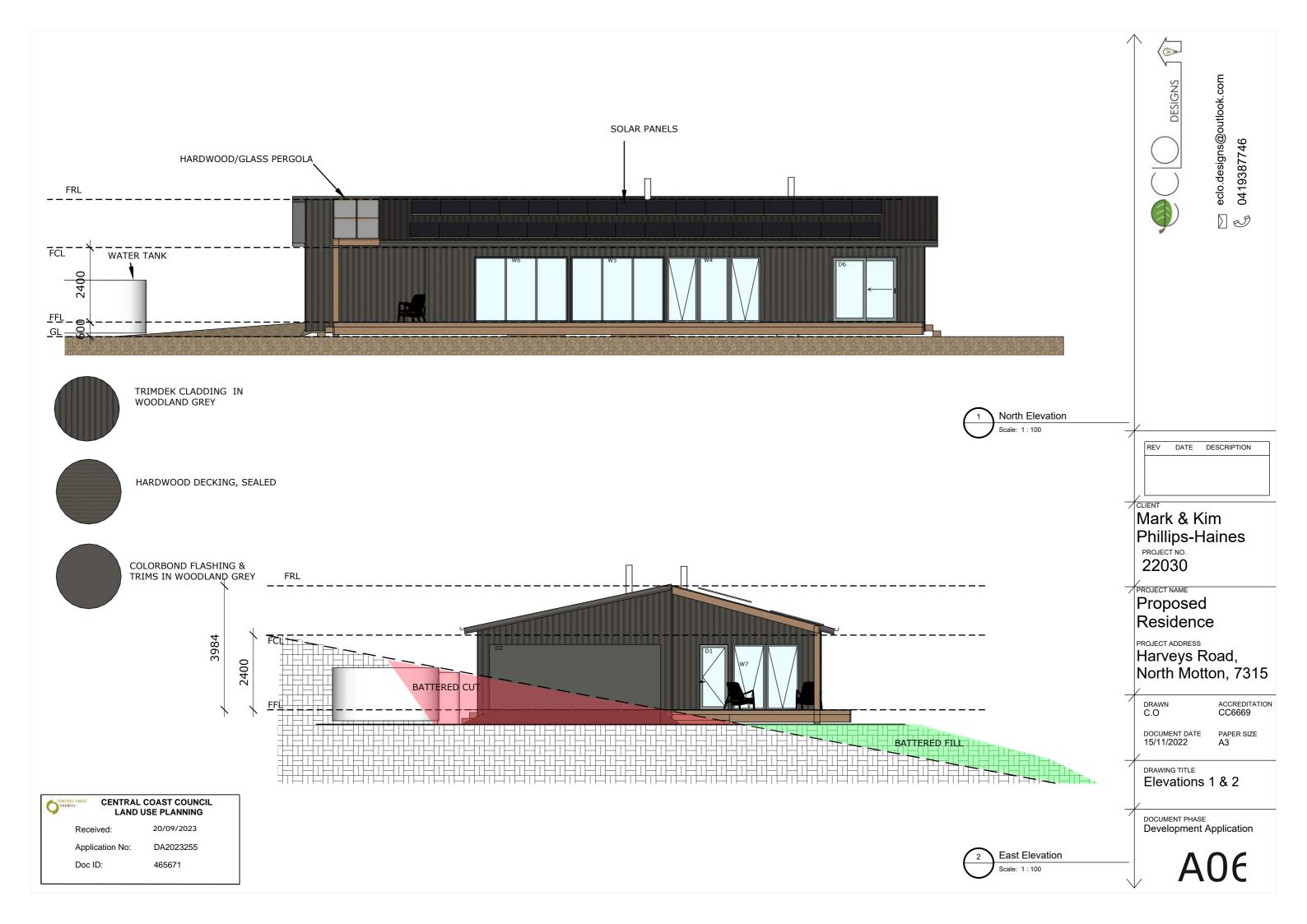


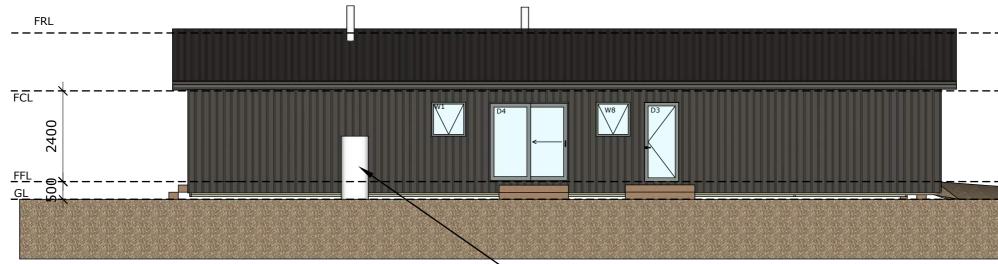














TRIMDEK CLADDING IN WOODLAND GREY

HARDWOOD DECKING, SEALED

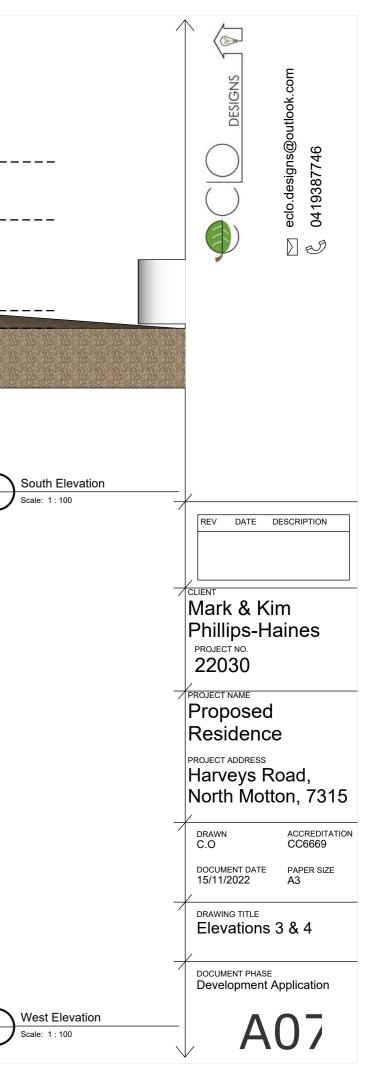


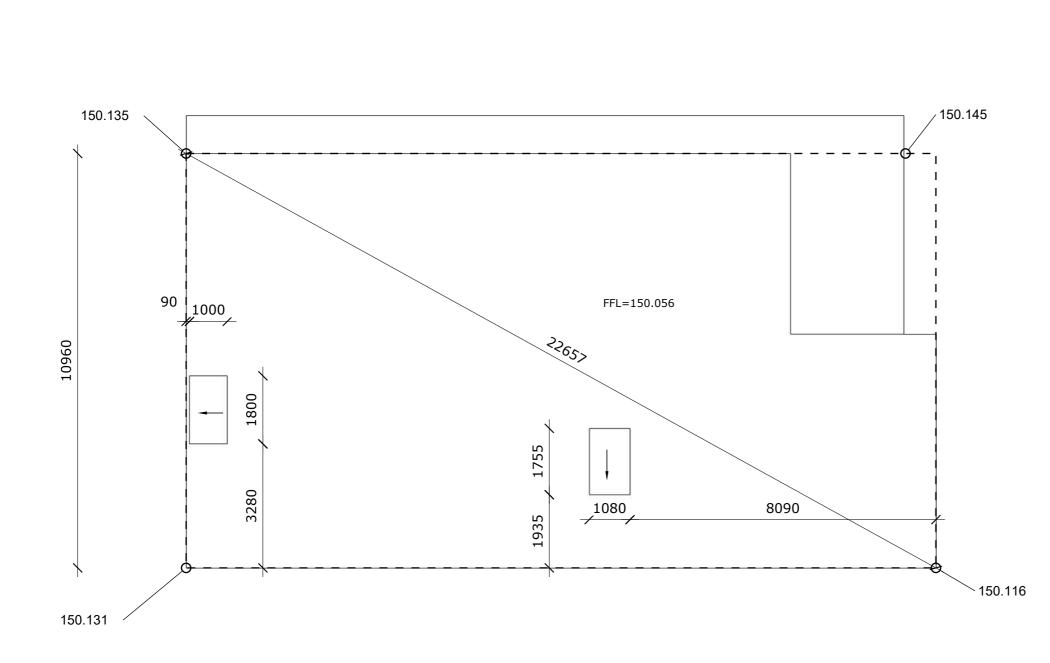
COLORBOND FLASHING & TRIMS IN WOODLAND GREY HOTWATER CLYINDER

COUNCIL	CENTRAL COAST COUNCIL LAND USE PLANNING		
Rece	ived:	20/09/2023	
Applie	cation No:	DA2023255	
Doc I	D:	465671	

4

3





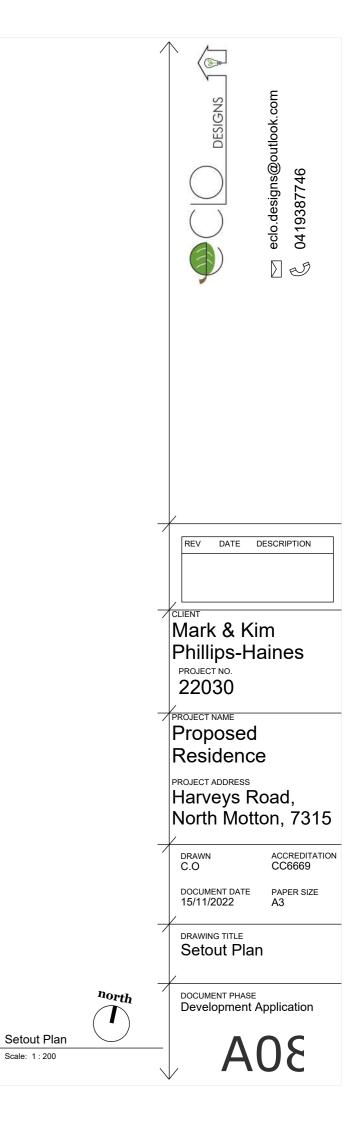
19830

STEPLESS SHOWER:

REBATE SHOWER BASES TO ENSURE APPROPRIATE FALL IS ACHIEVED 1:50. STAINLESS STEEL GRATE DRAIN RL -20 BELOW FLOOR LEVEL. (REFER TO A17 FOR WATER PROOFING DETAILS)

-





ROOF PLAN NOTES:

GUTTERS AND DOWNPIPES TO AS3500. MAXIMUM CENTRES FOR DOWNPIPES TO BE 12M.

ROOF STRUCTURE:

CUSTOM ORB ROOFING IN WOODLAND GREY & PERMIABLE WRAP TIMBER TRUSSES TO MANUFACTURES SPECIFICATIONS

EAVES VENTS WITH ALUMINUM MESH BACKING AT INTERVALS AS SHOWN (2500mm MAX. CENTRES). - EAVES VENT (200x400) EV

ROOF PLUMBING:

GENERIC QUAD 150 HIGH FRONT GUTTER IN WOODLAND GREY AND FASCIA

COLORBOND WOODLAND GREY CAPPINGS AND FLASHINGS

D.P. 100 x 50 UPVC SQUARE DOWNPIPES

NOTES:

WRAP MUST COMPLY WITH AS/NZS 4200 PARTS 1 AND 2.

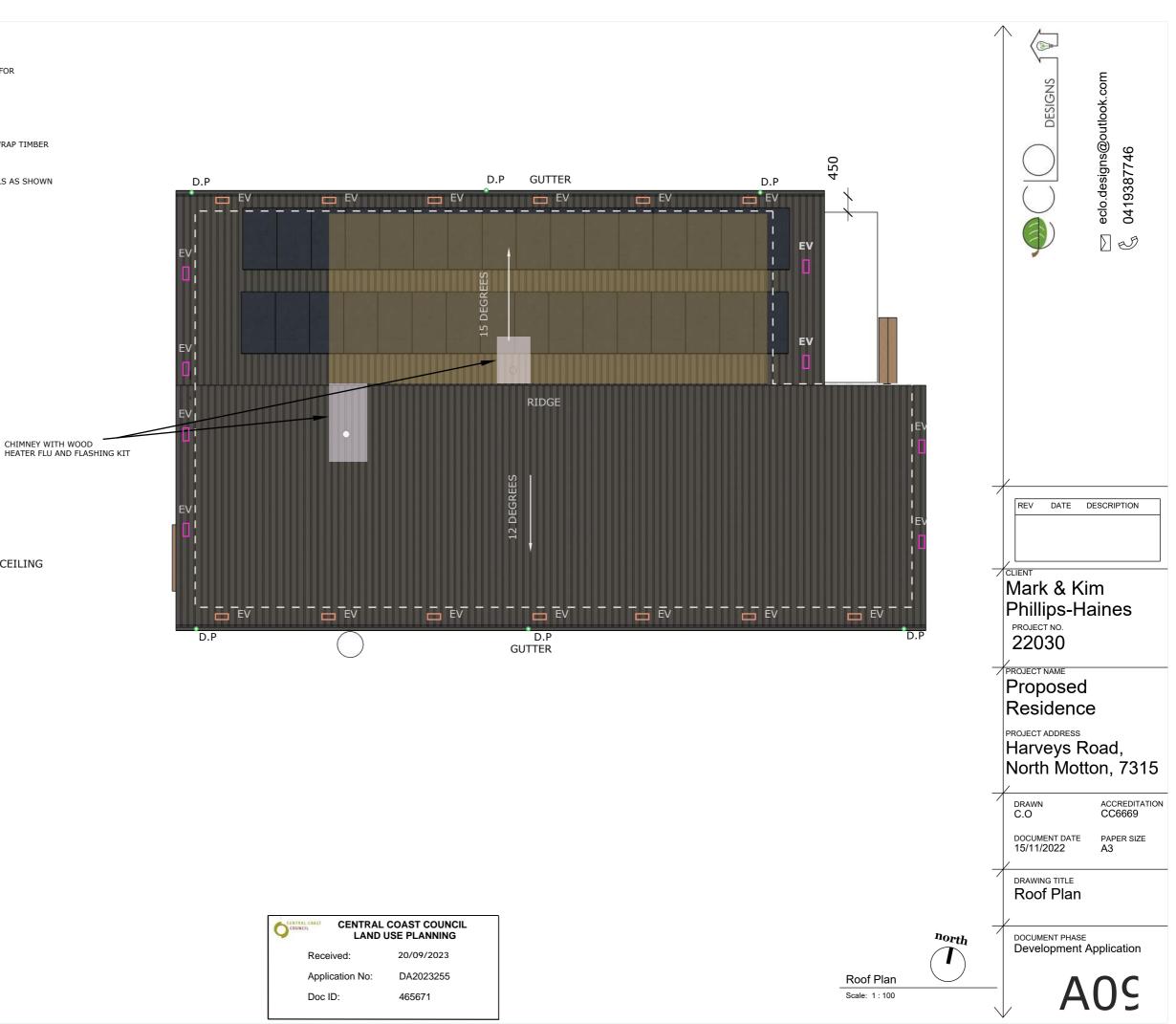
DOWNPIPES MUST NOT SERVE MORE THAN 12M OF GUTTER LENGTH FOR EACH DOWNPIPE.

ROOF CLADDING TO COMPLY WITH AS 1562.1.

ROOF DRAINAGE MUST COMPLY WITH: - PLUMBING CODE OF AUSTRALIA PART D1

- AS/NZS 3500.3 - BCA VOLUME 2 PARTS 3.1.2 AND 3.5.2. (DEEMED TO SATISFY PROVISIONS)

HYSPAN RAFTERS VAULTED CEILING



VENTILATION:

AS PER CBOS CONDENSATION GUIDELINES:

CEILING AREA/150 <16°

200m2/150= 1.33m2

75% SUPPLY =1m2 25% EXHAUST=0.33m2

CENTRAL COAST COUNCIL LAND USE PLANNING		
Received:	20/09/2023	
Application No:	DA2023255	
Doc ID:	465671	

LEGEND & NOTES

Stormwater line (100mm UPVC)

_ _ _ _ _ Sewer line (100mm UPVC)

Water line (100mm UPVC) _____

Install inspection openings at major bends for stormwater and all low points of downpipes.

All plumbing & drainage to be in accordance with local Council requirements.

Provide surface drain to back of bulk excavation to drain levelled pad prior to commencing footing excavation.

Services

The heated water system must be designed and installed with Part B2 of NCC Volume Three -Plumbing Code of Australia.

Thermal insulation for heated water piping must: a) be protected against the effects of weather and sunlight; and

b) be able to withstand the temperatures within the piping; and

c) use thermal insulation in accordance with AS/NZS 4859.1

Heated water piping that is not within a conditioned space must be thermally insulated as follows: 1. Internal piping

- a) All flow and return internal piping that is i) within an unventilated wall space
 - ii) within an internal floor between storeys; or
 - iii) between ceiling insulation and a ceiling

Must have a minimum R-Value of 0.2 (ie 9mm of closed cell

polymer insulation)

2. Piping located within a ventilated wall space, an enclosed building subfloor or a roof space

a) All flow and return piping

b) Cold water supply piping and Relief valve pipingwithin 500mm of the connection to central water heating system

Must have a minimum R-Value of 0.45 (ie 19mm of closed cell polymer insulation)

3. Piping located outside the building or in an unenclosed building sub-floor or roof space

a) All flow and return piping

b) Cold water supply piping and Relief valve pipingwithin 500mm of the connection to central water heating system

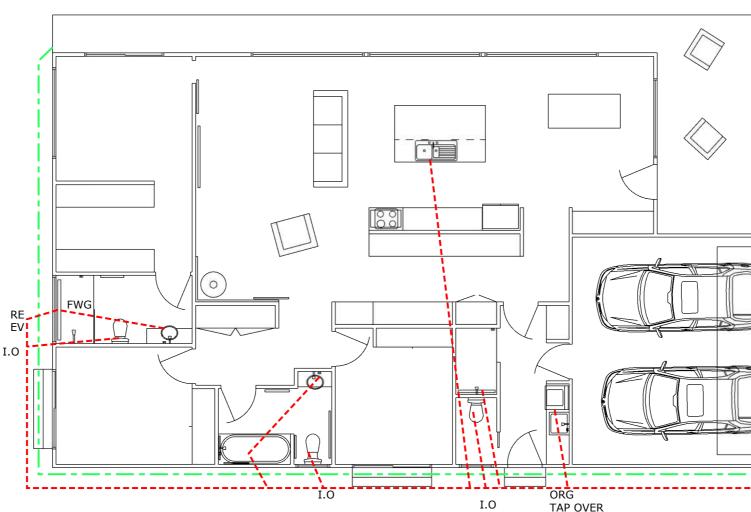
Must have a minimum R-Value of 0.6 (ie 25mm of closed cell polymer insulation)

Piping within an insulated timber framed wall, such as that passing through a wall stud, is considered to comply with the above insulation requirements.

Depth covers to AS3500.2

Vehicular traffic area all other locations

500mm 300mm



LEGEND:

AAV - AIR ADMITTANCE VALVE

- I.O INSPECTION SHAFT OPENING
- ORG OVERFLOW RELIEF GULLY
- DP - DOWN PIPE
- INSPECTION SHAFT I.S
- FC - FLEXIBLE CONNECTOR

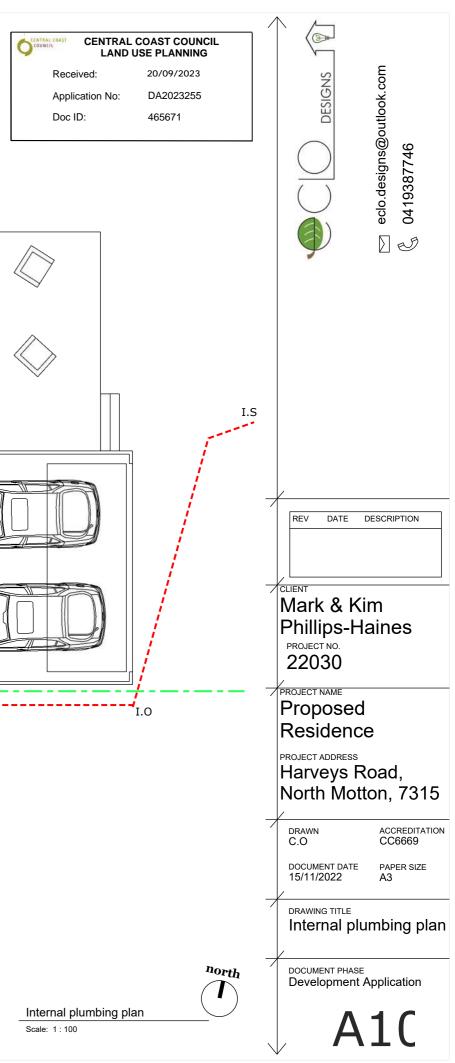
FWG - FLOOR WASTE GULLY (NO SMALLER THAN DN40 UNTRAPPED)

ORG rim to be minimum 150mm below lowest sanitary fitting.

ORG rim to be minimum 75mm

above outside gl.

Received Doc ID:

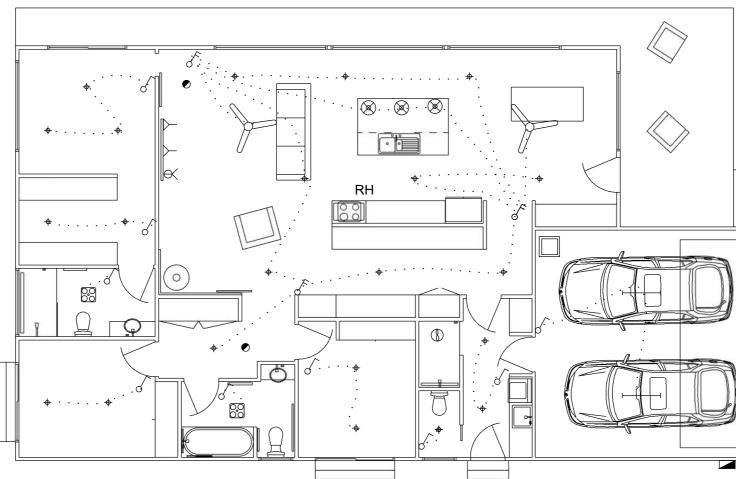


LEGEND

- 0.0 COMBINATION LIGHT, FAN, HEAT LAMP UNIT (4 LAMP)
- SMOKE ALARM (TO BE INTERCONNECTED IF MORE THAN ONE) ۲
- EXHAUST FAN ONLY (WITH INSECT SCREEN)
- \oplus SURFACE MOUNTED LIGHT
- DOWN LIGHT RECESSED +
- LED LIGHT STRIP SINGLE
- \otimes HANGING PENDANT LIGHT
- SWITCH, SINGLE б
- SWITCH, MULTI б
- ightarrowMULTI PURPOSE POWER POINT
- \succ TELEPHONE OUTLET
- \succ ETHERNET OUTLET
- еX TELEVISION OUTLET

CEILING FAN

- INSPECTION MAN HOLE
- METER BOX
- RANGE HOOD RH



LIGHT SCHEDULE

- 11W ILLUMINATION POWER LOAD В
- D.L 12W ILLUMINATION POWER LOAD
- 28W ILLUMINATION POWER LOAD F.L
- 15W ILLUMINATION POWER LOAD IXL (HEAT LAMPS NOT INCLUDED IN CALCULATION)

NOTES:

ALL FANS TO BE FITTED WITH BACK DRAUGHT DAMPERS/SHUTTERS DUCTED TO OUTSIDE

EXTERNAL SENSOR CONTROLLED BY A DAY LIGHT SENSOR OR HAVE AN AVERAGE LIGHT SOURCE EFFICENTCY NO LESS THAN 40 LUMENS/W

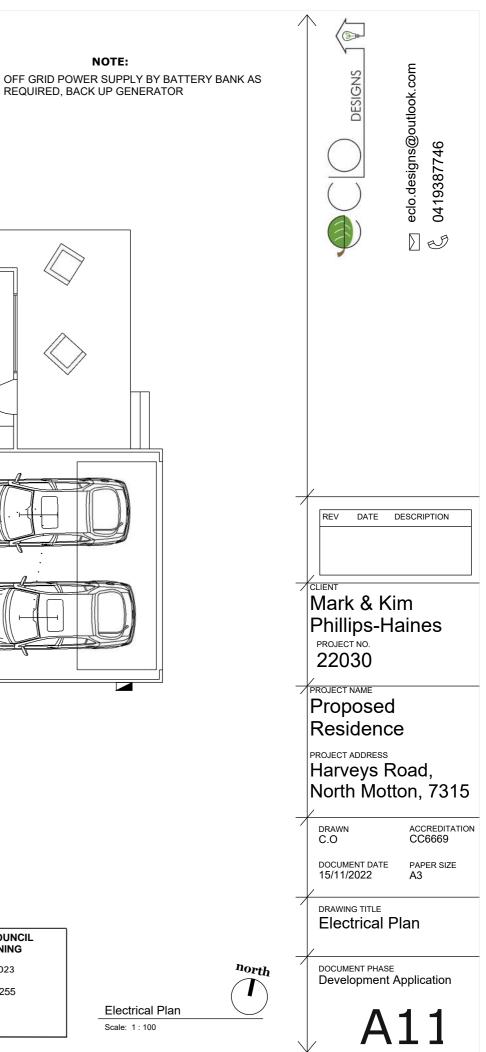
SMOKE ALARM TO AS 3786 AND NCC. HARD WIRED WITH BATTERY BACK UP, INTERLINKED

THIS LIGHTING SCHEDULE CORRESPONDS WITH THE NCC LIGHTING CALCULATOR.

DOWNLIGHTS NOT RECOMMENDED DUE TO AIR LEAKAGE AND LOSS OF INSULATION, IF DOWN LIGHTS ARE USED SPECIFY LED 645ETREW4R

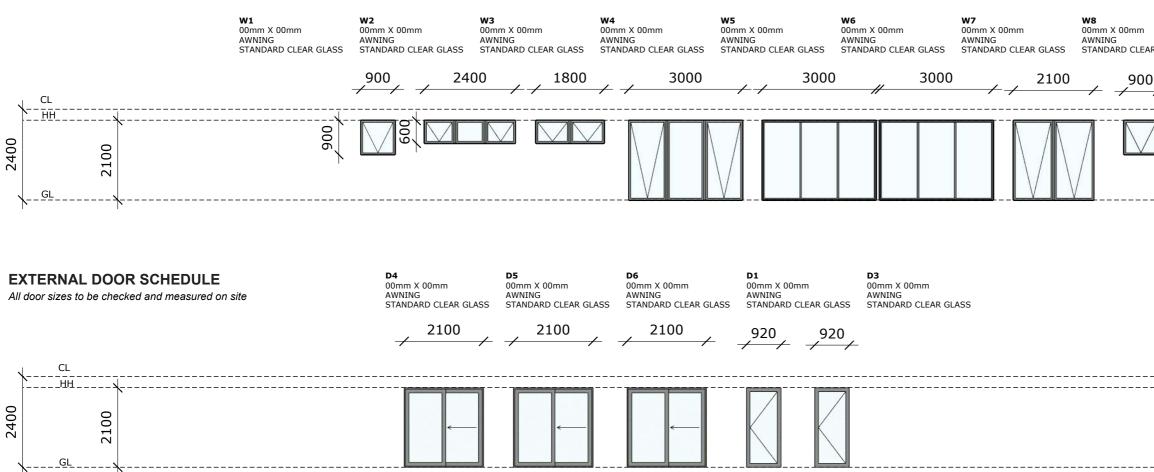
CENTRAL COAST	CENTRAL COAST COUNCIL LAND USE PLANNING						
Rece	ived:	20/09/2023					
Appli	cation No:	DA2023255					
Doc I	D:	465671					

NOTE: REQUIRED, BACK UP GENERATOR



BAL 29 ASSUMED TBC

WINDOW SCHEDULE All door sizes to be checked and measured on site



INTERNAL DOOR SCHEDULE

All door sizes to be checked and measured on site

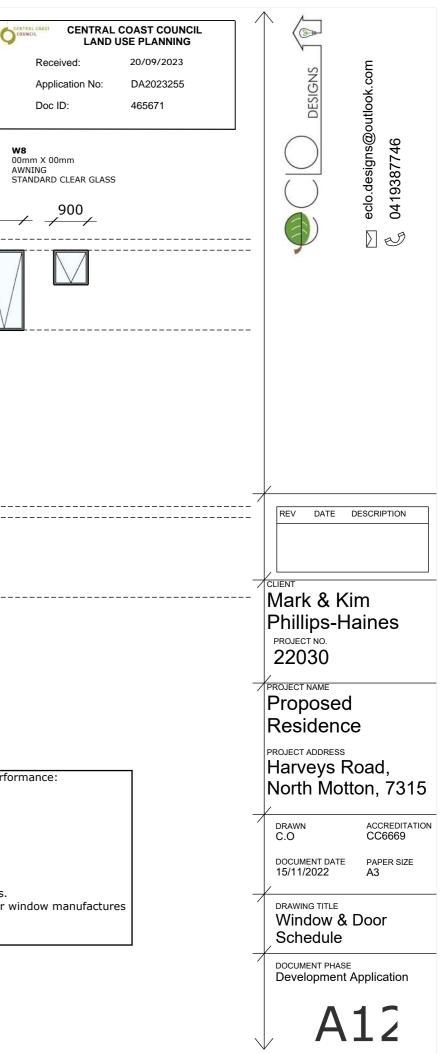
DOOR #	TYPE	HEAD HEIGHT	TOTAL DIMENSIONS	HARDWARE	INTERIOR ARCHITRAVE
D7	INTERNAL SWING DOOR	2100mm	2100mm x 920mm	LEVER HANDLE	67 x 18 PRE-PRIMED ARCHITRAVE PAINT FINISH
D8	INTERNAL SWING DOOR	2100mm	2100mm x 920mm	LEVER HANDLE WITH LOCK	67 x 18 PRE-PRIMED ARCHITRAVE PAINT FINISH
D9	INTERNAL SWING DOOR	2100mm	2100mm x 920mm	LEVER HANDLE & DRAFT SEAL	67 x 18 PRE-PRIMED ARCHITRAVE PAINT FINISH
D10	BARN SLIDING DOOR	2100mm	2100mm x 920mm	TOP TRACK RAIL	67 x 18 PRE-PRIMED ARCHITRAVE PAINT FINISH
D11	INTERNAL CAVITY SLIDER DOOR	2100mm	2100mm x 920mm	SIDE HANDLE	67 x 18 PRE-PRIMED ARCHITRAVE PAINT FINISH

Window	and	alazed	door	type	and	performance:

ALUMINIUM THERMALLY BROKEN DOUBLE GLAZED U VALUE- TBC SHGC- TBC

COLOR TBC

as per the star rating default windows. to be matched against WER values for window manufactures



NATURAL LIGHT & VENTILATION

PART 3.8.4 LIGHT: MINIMUM 10% OF THE FLOOR AREA OF A HABITABLE ROOM REQUIRED (NATURAL LIGHT) PART 3.8.5 VENTILATION: MINIMUM 5% OF THE FLOOR AREA OF A HABITABLE ROOM REQUIRED (AN EXHAUST FAN MAY BE USED FOR A SANITARY COMPARTMENT, LAUNDRY OR BATHROOM PROVIDE CONTAMINATED AIR DISCHARGES DIRECTLY TO THE OUTSIDE OF THE BUILDING BBY WAYS OF DUCTS)

FLYSCREENS TO BE FITTED TO ALL OPENABLE WINDOWS AND DOORS.

REFER TO ENERGY ASSESSMENT FOR GLAZING U-VALUE AND SHGC REQUIREMENTS SHOWER SCREENS 1800H SEMI-FRAMELESS SHOWER SCREENS TO COMPLY WITH BCA TABLE 3.6.5. & AS1288.

MINIMUM 4MM THICK GRADE A TOUGHENED SAFETY GLASS, LABELLED TO COMPLY WITH INDUSTRY STANDARDS.

ALL OPENINGS MUST BE ADEQUATELY FLASHED USING MATERIALS THAT COMPLY WITH AS/NZS 2904.

a) A DEVICE CAPABLE OF RESTRICTING THE WINDOW OPENING; OR

b) NOT PERMIT A 125MM SPHERE TO PASS THROUGH THE WINDOW OPENING OR SCREEN; AND
 b) RESIST AN OUTWARD HORIZONTAL ACTION OF 250N AGAINST THE:

- MARKING IN THE FORM OF AN OPAQUE BAND NOT LESS THAN 20MM IN HEIGHT;

- THE UPPER EDGE IS NOT LESS THAN 700MM ABOVE THE FLOOR; - THE LOWER EDGE IS NOT MORE THAN 1200MM ABOVE THE FLOOR.

OPAOUE BANDS

BENEATH.

READILY VISIBLE AS FOLLOWS:

FLASHINGS TO WALL OPENINGS

b) A SCREEN WITH SECURE FITTINGS. THE DEVICE OR SCREEN MUST:

WINDOW RESTRAINED BY A DEVICE; OR

SCREEN PROTECTING THE OPENING: AND

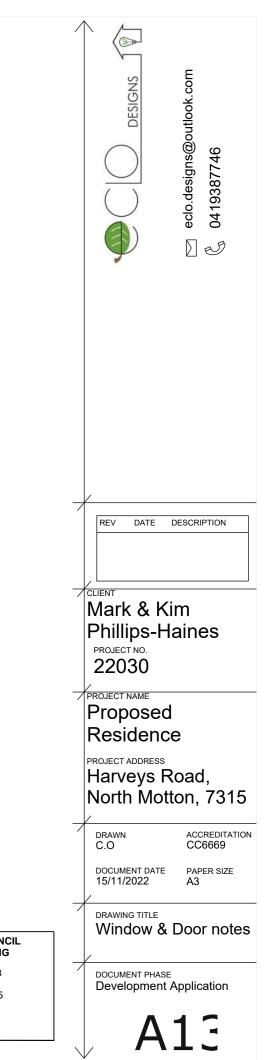


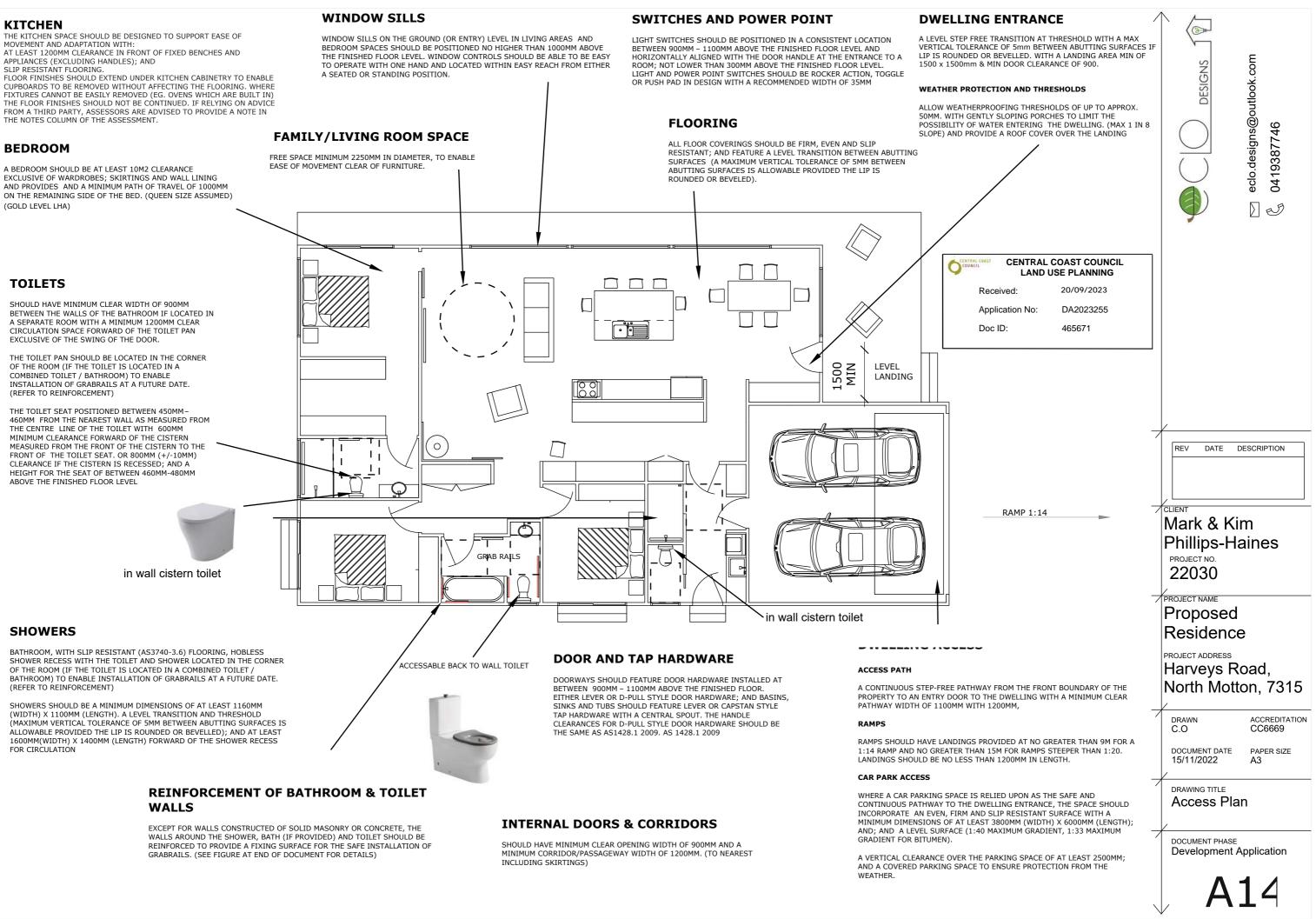
c) HAVE A CHILD RESISTANT RELEASE MECHANISM IF THE SCREEN OR DEVICE IS ABLE TO BE REMOVED, UNLOCKED OR OVERRIDDEN.

ALL GLAZED WINDOW & DOOR ASSEMBLIES IN EXTERNAL WALLS TO COMPLY WITH AS 2047. ALL OTHER GLASS TO COMPLY WITH AS 1288.

WHERE GLAZED DOORS OR SIDE PANELS ARE CAPABLE OF BEING MISTAKEN FOR A DOORWAY OR OPENING, THE GLASS MUST BE MARKED TO MAKE IT

REFER TO SECTIONS FOR WINDOW HEAD AND SILL DETAILS. FLASHING TO BE INSTALLED WITH GLAZING MANUFACTURER'S SPECIFICATIONS FOR BRICK VENEER CONSTRUCTION. PROTECTION OF OPENABLE WINDOWS. A WINDOW OPENING MUST BE PROVIDED WITH PROTECTION, IF THE FLOOR BELOW THE WINDOW IN A BEDROOM IS 2M OR MORE ABOVE THE SURFACE





Bushfire Hazard Report



AUTHOR DETAILS:

Accredited Assessor: Contact Phone Number: Email: Accreditation Details: Report Date: Report Reference:

PROPERTY DETAILS:

Location: Applicant: Proposal: Zoning: Title Reference: PID: Jayne Newman 0438 132 162 planning4bushfire@gmail.com BFP – 158 (Scope 1, 2, 3B & 3C) 29 March 2024 24037-B

Harveys Road, North Motton Mark and Kim Phillips-Haines Single Dwelling Rural CT: 131560/2 1906437

CENTRAL COAST	CENTRAL COAST COUNCIL LAND USE PLANNING						
Recei	ved:	16/04/2024					
Applic	ation No:	DA2023255					
Doc II	D:	483177					



Table of Contents

1.	Scope	e of Assessors Accreditation	3
2.	Discla	aimer	3
3.	Execu	utive Summary	4
4.	Intro	duction	4
5.	Site D	Description	4
6.	Propo	osed Development	4
7.	Busht	fire Hazard Assessment	5
	7.1	Surrounding Lot Analysis (photos taken 12 March 2024)	5
	7.2	Vegetation Classification	7
	7.3	Bushire Attack Level	8
8	Bushi	fire Protection Measures	9
	8.1	Structure	9
	8.2	Access	9
	8.3	Water Supply	9
	8.4	Hazard Management Area 1	0
8.	Statu	tory Compliance1	1
9.	Concl	lusion 1	1
10.	Appe	ndix1	1
	a)	Eclo Designs - Project Number 220301	1
	b)	Bushfire Hazard Management Plan1	1
	c)	Assessable Item (Form 55) 1	1

Author – Jayne Newman BHP-158

1. Scope of Assessors Accreditation

Jayne Newman (BFP- 158) is accredited by the Chief Office of the Tasmanian Fire Service under Section 60B of the Fire Service Act 1979 for the following scope of works:

- 1. Certify a Bushfire Hazard Management Plan for the purpose of the Building Act 2016.
- 2. Certify an Exemption from a Bushfire Hazard management Plan for the purposes of the Building Act 2016 or the Land Use and Approvals Act 1993.
- 3B. Certify a Bushfire Hazard management Plan meets the Acceptable Solutions for small subdivisions (10 lots or less) for the purposes of the Land Use Planning and Approvals Act 1993.
- 3C. Certify a Bushfire Hazard Management Plan meets the Acceptable Solutions for large subdivisions (less than 10 lots) for the purposes of the Land Use Planning and Approvals Act 1993.

2. Disclaimer

This document has been prepared for the sole use of the client and for a specific purpose, as expressly stated in the document. Planning 4 Bushfire undertakes no duty nor accepts any responsibility to any third party not being the intended recipient of this document. The information contained in this document has been carefully compiled based on the clients' requirements and Planning 4 Bushfire experience, having regard to the assumptions that Planning 4 Bushfire can reasonably be expected to make in accordance with sound professional principles. Planning 4 Bushfire may also have relied on information provided by the client and/or other external parties to prepare this document, some of which may not have been verified. Subject to the above conditions, Planning 4 Bushfire recommends that this document should only be transmitted, reproduced or disseminate in its entirety.

Bushfires in Tasmania are an unpredictable natural phenomenon and preparing a Bushfire Hazard Management Plan increases your chances of defending your property and assist in the protection the people who frequent it. The Fire Hazard Management Plan in no way guarantees immunity from bushfire in or around your property or the effects thereof.

Any measures implemented based on the advice from Planning 4 Bushfire, is offered as potential methods of reducing your properties risk of fire damage only and is not to be relied upon as a total solution. It in no way guarantees that any or all buildings on site will survive the effects of a bushfire nor does it guarantee the safety and security of any individuals who frequent the property.

In the event that any advice or other services rendered by Planning 4 Bushfire constitutes a supply of service to a consumer under the Trade Practices Act 1974 (as amended), then Planning 4 Bushfire's liability for any breach of any conditions or warranties implied under the Act shall not be excluded but will be limited to the cost of having the advice or services supplied again.

Nothing in this Disclaimer affects any rights or remedies to which you may be entitled under the Trade Practices Act 1974 (as amended). Each paragraph of this disclaimer shall be deemed to be separate and severable from each other. If any paragraph is found to be illegal, prohibited or unenforceable, then this shall not invalidate any other paragraph.

3. Executive Summary

This report together with the appendices have been prepared to provide advice relative to the Building Act 2016 AS 3959-2018 and the Directors Determination – Bushfire Hazard Areas (v1.1). The report findings dictate a BAL Level of 19 for the single dwelling, which is an acceptable level of risk.

4. Introduction

This report has been prepared on behalf of the client to provide a bushfire hazard management assessment for the proposed single dwelling.

The site is zoned Rural with all surrounding land also zoned Rural which then adjoins Environmental Management further south and west.

This report has been prepared with assessment against the requirements detailed within Directors Determination – Bushfire Hazard Areas (v1.1). and identifies the bushfire attack level and any bushfire hazard management areas in accordance with Australian Standard AS 3959-2018.

5. Site Description

Proposal	Single Dwelling
NCC Building Class	1A
Address	Harveys Road, North Motton
Certificate of Title	CT: 131560/2
Land area	17.54ha
Zoning	Rural
Current Land Use	Existing rural sheds

The site is 17.54ha in size containing some small sheds on the entrance to the property.

6. Proposed Development

The proposal relates to a new single dwelling comprising of three bedrooms, bathroom, mud room, double garage and open plan kitchen, living and dining room adjoining a northern and eastern facing deck. The dwelling will be clad in a trimdek with colorbond roofing. The dwelling is sited approximately 100 metres from the closest boundary to the north. The proposed height is a maximum of 3.984 metres being cut into the embankment.

7. Bushfire Hazard Assessment

7.1 Surrounding Lot Analysis (photos taken 12 March 2024).



Image 3 – North: Forest vegetation. This area has previously been cleared, being at regrowth stage.



Image 4 – East: – Cleared land then forest vegetation.



Image 5 – South: Forest vegetation.



Image 6 – West: Forest vegetation.

7.2 Vegetation Classification



Image 7: Source Listmap - TasVeg 4.0

The layer covering the development site is a mix of (WOB) Wet eucalyptus obiqua forest (shown in green) and (FAG) agricultural land, which is not regrown into modified land, being regrowth Forst.

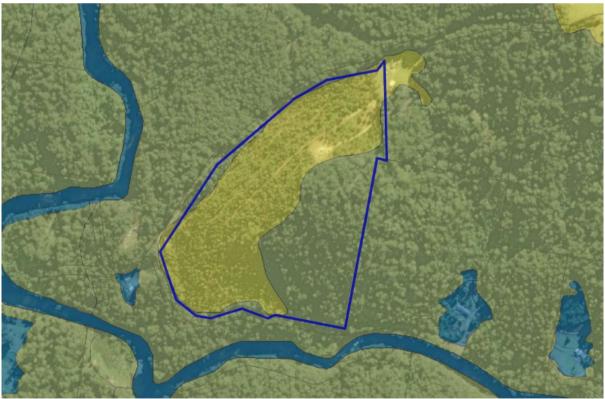


Image 8: Source Listmap - TasVeg 4.0 Fire Attributes

Land shown in yellow has a medium to high flammability and low sensitivity, while land shown in green has a medium to high flammability and high sensitivity.

Fire Danger Index FDI 50 √ Assessment of vegetation located within 100m of the site

Vegetation Classification	North √ North-East □	South √ South-West □	East √ South-East □	West √ North-West □
Group A Forest	√ 51m min	√ 23m min	√ 23m min East 34m North/East	√ 51m min
Exclusions (where applicable	(a)(b)(c)(d)(e)(f)	(a)(b)(c)(d)(e)(f)	(a)(b)(c)(d)(e)(f)	(a)(b)(c)(d)(e)(f)
Distances to classified vegetation	A clearance of 51 metres minimum is required and must be maintained as a HMA.	A clearance of 23 metres minimum is required and must be maintained as a HMA.	A clearance of 23 metres minimum to the East and 34 metres minimum to the North/East is required and must be maintained as a HMA.	A clearance of 51 metres minimum is required and must be maintained as a HMA.

Effective Slope

Effective slope	Upslope							
	Upslope/0°		Upslope/0°		Upslope 0°	\checkmark	Upslope 0°	
			D	ownslo	ре			
Slope under the	>0 to 5°		>0 to 5°	\checkmark	>0 to 5°		>0 to 5°	
classified vegetation	>5 to 10°		>5 to 10°		>5 to 10°	\checkmark	>5 to 10°	
	>10 to 15°		>10 to 15°		>10 to 15°		>10 to 15°	
	>15 to 20°	\checkmark	>15 to 20°		>15 to 20		>15 to 20°	
BAL value	19		19		19		19	

A lidar was undertaken to determine slope due to potentially exceeding 20 degrees to the north. This came back with a slope of 19.9 degrees, therefore suitable for assessment under the deemed to satisfy provisions. It is also noted that previous clearing and track construction in this area has reduced the slope from that identified on the lidar. The owner also intends on providing fill in this area to further reduce the slope during clearance. This will provide greater reduction in slope in a northerly direction.

The applicable Bushfire Attack Level is: BAL – 19

8 Bushfire Protection Measures

8.1 <u>Structure</u>

The proposed single dwelling is to be constructed to a BAL 19 standard as per AS 3959-2018 Section 6.

8.2 <u>Access</u>

Element B - The access length to the proposed dwelling exceeds 30 metres therefore following applies.

The following design and construction requirements apply to property access;

- (a) All-weather construction;
- (b) Load capacity of at least 20 tonnes, including for bridges and culverts;
- (c) Minimum carriageway width of 4 metres;
- (d) Minimum vertical clearance of 4 metres;
- (e) Minimum horizontal clearance of 0.5 metres from the edge of the carriageway;
- (f) Cross falls of less than 3_{\circ} (1:20 or 5%);
- (g) Dips less than 7o (1:8 or 12.5%);
- (h) Curves with a minimum inner radius of 10 metres;

(i) Maximum gradient of $15_{\circ}\,(1:3.5~\text{or}~28\%)$ for sealed roads, and 10o (1:5.5 or 18%) for unsealed roads; and

(j) Terminate with a turning area for fire appliance provided by one or the following;

(i) A turning circle with a minimum outer radius of 10 metres;

- (ii) A property access encircling the building; or
- (iii) A hammerhead "T" or "Y" turning head 4 metres wide and 8 metres long.

<u>Element C – Property access length is 200 metres or greater, therefore the following design and construction requirements apply.</u>

- (a) The Requirements for B above; and
- (b) Passing bays of 2 metres additional carriageway width and 20 metres length provided every 200 metres.

8.3 <u>Water Supply</u>

A static water supply is required and must comply with the following. Element \underline{A} - The following requirements apply:

- (a) The building area to be protected must be located within 90 metres of the firefighting water point of a static water supply; and
- (b) The distance must be measured as a hose lay, between the firefighting water point and the furthest part of the building area.

Element B - A static water supply;

- (a) May have a remotely located offtake connected to the static water supply;
- (b) May be a supply for combined use (fire fighting and other uses) but the specific minimum quantity of fire fighting water must be available at all times;
- (c) Must be a minimum of 10,000 per building area to be protected. This volume of water must not be used for any other purpose including fire fighting sprinkler or spray systems;
- (d) Must be metal, concrete or lagged by non-combustible materials if above ground; and
- (e) If a tank can be located so it is shielded in all directions in compliance with Section 3.5 of AS 3959-2018, the tank may be constructed of any material provided that the lowest 400mm of the tank exterior is protected by;
 - (i) Metal;
 - (ii) Non-combustible material; or
 - (iii) Fibre-cement a minimum of 6mm thickness."

<u>Element C</u> - Fittings and pipework associated with a firefighting water point for a static water supply must:

- (a) Have a minimum nominal internal diameter of 50mm;
- (b) Be fitted with a valve with a minimum nominal internal diameter or 50mm;
- (c) Be metal or lagged by non-combustible materials if above ground;
- (d) Where buried, have a minimum dept of 300mm;
- (e) Provide a DIN or NEN standard forged Storz 65mm coupling fitting with a suction washer for connection to firefighting equipment;
- (f) Ensure the coupling is accessible and available for connection at all times;
- (g) Ensure the coupling is fitting with a blank cap and securing chain (minimum 220mm length);
- (h) Ensure underground tanks have either an opening at the top of not less than 250mm diameter or a coupling compliant with this Table; and
- (i) Where a remote offtake is installed, ensure the offtake is in a position that is;
 - (i) Visible;
 - (ii) Accessible to allow connection by firefighting equipment;
 - (iii) At a working height of 450-600mm above ground level; and
 - (iv) Protected from possible damage, inducing damage by vehicles.

<u>Element D -</u> The firefighting water point for a static water supply must be identified by a sign permanently fixed to the exterior of the assembly in a visible location. The sign must:

(a) comply with water tank signage requirements within AS 2304:2019; or

(b) comply with the TFS Water Supply Signage Guideline.

<u>Element E -</u>Hardstand

A hardstand are for fire appliances must be provided:

- (a) No more than three metres from the firefighting water point, measured as a hose land (including the minimum water level in dams, swimming pools and the like);
- (b) No closer than six metres from the building area to be protected;
- (c) With a minimum width of three metres constructed to the same standard as the carriageway; and
- (d) Connected to the property access by a carriageway equivalent to the standard of the property access.

Note:

1. If for some reason the fire fighting tank is to be located in an alternate location, it must still comply with the requirements of Table 3B of the Directors Determination taking into consideration Table 2, to still comply with the requirements of this report.

8.4 Hazard Management Area

The Hazard Management Area as depicted on the plan, is to be implemented and must be maintained to a low threat vegetation standard having grass managed to a height of less than 100mm for the life of the building prior to the issuance of an occupancy certificate.

- Additional recommendations are made to achieve and maintain a minimal fuel level as follows;
 - a. Keep gutters and roof spaces free from debris.
 - b. Locate paths, paved areas and driveways near the dwelling.
 - c. Do not store fire wood within 6 metres of buildings.
 - d. Do not use pine bark or flammable garden mulch against building facades or specifically, windows located within 400mm of the ground.
 - f. If trees are planted within the bushfire hazard management area, consider using low flammable trees and shrubs as outlined within Fire Resisting Garden Plans for urban fringe and rural areas available online via www.fire.tas.gov.au.

8. Statutory Compliance

Deemed to Satisfy requirement	Compliance
Table 1 Construction	Construction requirements are to be to a BAL-19 standard in accordance with AS 3959-2018 and the Directors Determination – Bushfire Hazard Areas v 1.1.
Table 2 Requirements for Property Access	The access shown within the BHMP can achieve compliance with Table 2. The existing access will need to be upgraded to provide a horizontal and vertical clearance of 4 metres together with an additional 0.5 metre clearance either side of the carriageway and a hardstand area shown next to the fire fighting water supply. The road provides for an entry and exit but will still require a passing bay as shown.
Table 3B Requirements for Static Water Supply	A static water supply of 10,000 litres is required as shown on the Hazard Management Plan. The water supply provides a direct offtake next to the driveway hardstand area.
Table 4 Requirements for Hazard Management Area	The Bushfire Hazard Management Plan dictates a Hazard Management area providing for the minimum area to be maintained to achieve a BAL 19, compliant with Table 2.6 of AS 3959-2018.
Table 5 Requirements for Emergency Planning	Not applicable

9. Conclusion

This assessment provides a BAL of 19 for Harveys Road, North Motton. To comply with this report, the single dwelling must be constructed in accordance with Building Standards dictated by AS 3959-2018.

This report dictates requirements for construction, hazard management areas, access (including passing bays) and water supply, which must be implemented/installed prior to the issuance of an occupancy certificate and maintained for the life of the building.

Subject to implementing the Bushfire Hazard Management Report, the proposed single dwelling will comply with deemed to satisfy Tables 1, 2, 3B and 4 of the Directors Determination – Bushfire Hazard Areas (v1.1).

10. Appendix

- a) <u>Eclo Designs Project Number 22030</u>
- b) Bushfire Hazard Management Plan
- c) Assessable Item (Form 55)

CONTOURS AT 10m INTERVALS

SITE PLAN LEGEND & NOTES:

GENERAL NOTES:

DURING CONSTRUCTION SOIL AND WATER IS TO BE APPROPRIATELY MANAGED. THIS INCLUDES THE PROVISION OF SILT FENCING, FILTER SCREENS OR DEDICATED SILT TRAPS TO PREVENT DISCHARDGE OF GRAVEL, SOIL OR OTHER DEBRIS TO ANY EXISTING WATER COURSE OR ADJOINING PROPERTY DURING THE CONSTRUCTION PROCESS.

EXCAVATION:

ALLOW FOR BULK EXCAVATION WHERE REQUIRED AND ALL EXCAVATION, FILLING, BACK FILLING AND CONSOLIDATION REQUIRED FOR THE FOOTINGS AND SLAB, RETAIN ALL ACCESS AND SERVICES INDICATED. MAKE GOOD.

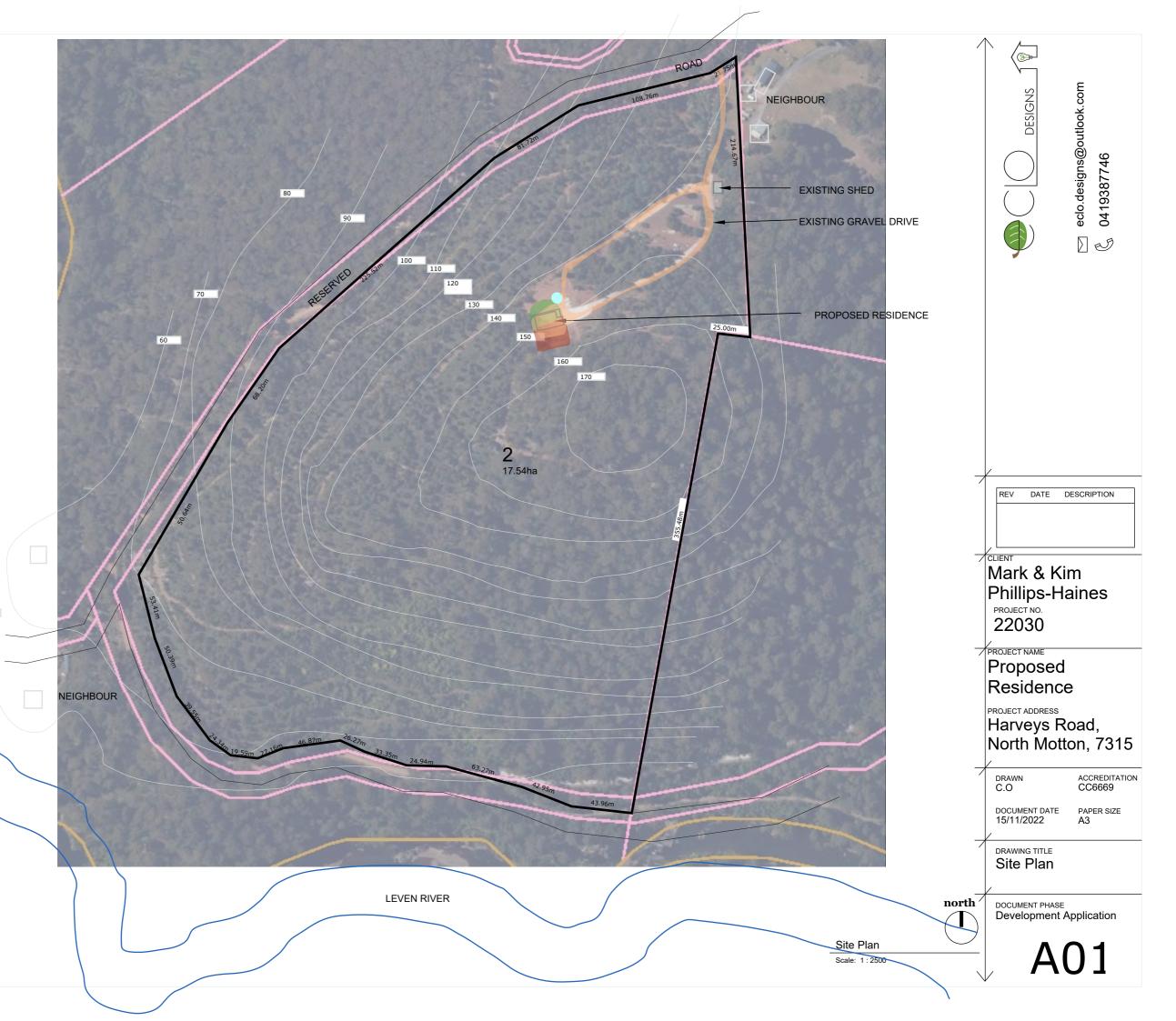
SETTING OUT:

THE CLIENT IS RESPONSIBLE FOR VERIFYING THE BOUNDARY PEGS ARE IN THE CORRECT LOCATION, MARKED AND CLEARLY VISIBLE FOR THE BUILDER. THE BUILDER SHALL ACCURATELY SET-OUT THE WORKS AND VERIFY ALL DIMENSIONS AND LEVELS BEFORE COMMENCING ANY WORKS. AND SHALL MAKE GOOD AT HIS OWN EXPENSE ANY ERRORS ARISING FROM INACCURACIES OF THE SETOUT.

PROTECTION WORK:

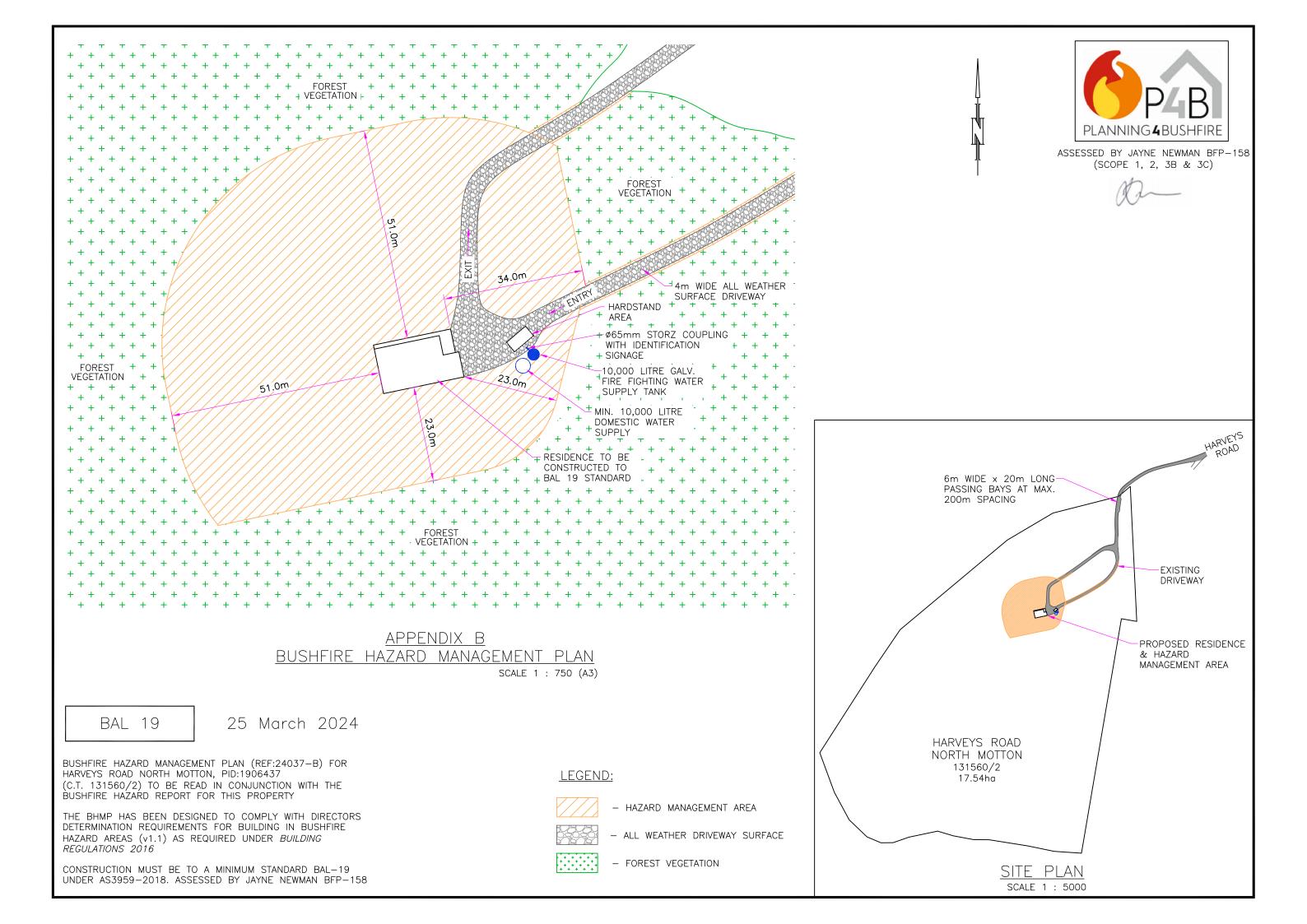
(SECTION 121 OF THE BUILDING ACT) IF EXCAVATION IS TO A LEVEL BELOW THAT OF THE ADJOINING OWNER'S FOOTINGS, ALONG THE TITLE BOUNDARY OR WITHIN 3 METRES OF A BUILDING BELONGING TO AN ADJOINING OWNER, THE BUILDER MUST (AS A MINIUMUM) PROVIDE AND MAINTAIN A GUARD TO SUPERVISE THE EXCAVATION. ADJOINING OWNER TO BE NOTIFIED USING FORM 6 (BUILDING AND PROTECTION WORK NOTICE) BY THE BUILDING SURVEYOR.

50



NO SERVICES AVALIABLE FOR THE SITE

SITE SERVICES:



CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

To:	Mark and Kim Phillips-Haines			Owner /Agent		F F
	1 Walker Street			Address	Form	55
	ULVERSTONE TAS 7315			Suburb/postcode	Э	
Qualified perso	on details:					
Qualified person:	Jayne Newman					
Address:	1175 Oldina Roa	ıd		Phone No:	0438 2	132 162
	Oldina		7325	Fax No:		
Licence No:	BFP-158	Email address:	plannir	ng4bushfire@	gmail.c	om
Qualifications and Insurance details:	Accredited to rep hazards under Pa Service Act 1979	art IVA of the Fi	Dire	cription from Columr ctor's Determination vualified Persons for s	- Certificate	
Speciality area of expertise:	Analysis of hazards in bushfire- prone areas(description from Column 4 of the Director's Determination - Certificates by Qualified Persons for Assessable Items)					
Details of work	K:					
Address:	Harveys Road,				Lot No:	2
	North Motton		7316	Certificate of	f title No:	131560
The assessable item related to this certificate:	Bushfire Hazard for a new single o	5	•	(description of th certified) Assessable item - a material; - a design - a form of co - a document - testing of a system or p - an inspectio performed	n includes – Instruction Component Jumbing sys	, building stem
Certificate details:						
Certificate type:	Bushfire Hazard		Schedi Determ	ption from Column 1 Jle 1 of the Director's ination - Certificates ed Persons for Asse 1)	s by	
This certificate is in relation to the above assessable item, at any stage, as part of - <i>(tick one)</i> building work, plumbing work or plumbing installation or demolition work:						

a building, temporary structure or plumbing installation:

In issuing this certificate the following matters are relevant -

	Bushfire Hazard Report – Harveys Road, North Motton (Jayne Newman Planning 4 Bushfire, 29 March 2024).
	Bushfire Hazard Management Plan – Harveys Road, North Motton – 25 March 2024.
	Eclo Designs – Site Plan – Project no 22030 – dated 15.11.2022
Relevant calculations:	
References:	Australian Standard 2050 2019 Construction of buildings in bushfire
References.	Australian Standard 3959-2018 Construction of buildings in bushfire- prone areas
	Director's Determination – Bushfire Hazard Areas (v1.1)
	Substance of Certificate: (what it is that is being certified)

The bushfire hazard management plan prescribes requirements for construction, access, water supply and hazard management areas. All requirements must be implemented prior to the issuance of an occupancy permit.

The proposed single dwelling must be designed and constructed to a minimum standard of BAL-19, under AS 3959-2018.

Subject to implementation of the bushfire hazard management plan the proposed Class 1a building will comply with the Deemed-to-Satisfy Provisions of the *Director's Determination – Bushfire Hazard Areas.*

Scope and/or Limitations

- 1. The effectiveness of the measures and recommendations made in the abovementioned documentation are dependent on their implementation and maintenance for the life of the subject building.
- 2. The assessed Bushfire Attack Level is correct at the time of certification. No liability can be accepted for the actions of other parties that compromise the effectiveness of the recommended hazard management area or construction standards.
- 3. Whilst implementation of the bushfire hazard management plan will enhance the likelihood of the building work surviving a bushfire, no guarantee is made that the building will survive every bushfire event.

I certify the matters described in this certificate.

Qualified person:Signed:Certificate No:Date:Qualified person:Date:29/03/2024



CENTRAL COAST COUNCIL LAND USE PLANNING					
Received:	20/09/2023				
Application No:	DA2023255				
Doc ID:	465674				

LANDSLIDE RISK ASSESSMENT, SITE CLASSIFICATION & ONSITE WASTEWATER ASSESSMENT AND DESIGN

Mr Mark & Mrs Kim Phillips-Haines

Harveys Road, North Motton

GL20322Ab 28 August 2020



Geoton Pty Ltd ABN 81 129 764 629 PO Box 522 Prospect TAS 7250 Unit 24, 16-18 Goodman Court Invermay TAS 7248 Tel (+61) (3) 6326 5001 www.geoton.com.au

28 August 2020

Reference No. GL20322Ab

Mr Mark & Mrs Kim Phillips-Haines 1 Walker Street Ulverstone TAS 7315

Dear Sir and Madam

RE: Landslide Risk Assessment, Site Classification & Onsite Wastewater Assessment and Design Harveys Road, North Motton

We have pleasure in submitting herein our report detailing the results of the geotechnical investigation conducted at the above site.

Should you require clarification of any aspect of this report, please contact Matthew Street or the undersigned on 03 6326 5001.

For and on behalf of Geoton Pty Ltd

Tony Barriera Director

CONTENTS

1	INTRODUCTION	3
2	BACKGROUND	3
2.1	Geology	3
2.2	Landslide Hazards	3
3	FIELD INVESTIGATION	3
4	SITE CONDITIONS	4
4.1	Site Description	4
4.2	Subsurface Conditions	4
4.3	Laboratory Testing	5
5	GEOLOGICAL MODEL	5
6	LANDSLIDE RISK ASSESSMENT	5
6.1	Incremental Landslide Hazards	6
6.2	Landslide Consequences	7
6.3	Landslide Risk to Property	7
6.4	Landslide Risk to Life	7
7	DISCUSSION AND RECOMMENDATIONS	8
8	SITE CLASSIFICATION	9
9	FOUNDATIONS	9
10	WIND CLASSIFICATION	10
11	ADDITIONAL WORK	10
12	EFFLUENT DISPOSAL	10

12.1	Permeability of Soil and Soil Category	11
12.2	Disposal and Treatment Method	11
12.3	Design Loading Rate	11
12.4	Absorption Trench System	11
12.5	Setbacks	12
12.6	Wastewater Recommendations	12

Limitations of Report

Tables

Table 1: Summary of Laboratory Test Results
Table 2: Summary of Estimated Pre-existing Landslide Hazard
Table 3: Summary of Consequences for Different Landslide Scenarios
Table 4: Summary of Assessed Landslide Risks to Property (AGS 2007c)
Table 5: Landslide Risk to Life for Person Most at Risk

Figures

Figure 1: Site Plan Figure WW-02 – Typical Trench Section Site Photographs

Appendices

- Appendix A Test Pit Logs & Explanation Sheets
- Appendix B Qualitative Terminology for Use in Assessing Risk to Property
- Appendix C Some Guidelines for Hillside Construction
- Appendix D Certificate Forms

1 INTRODUCTION

A limited scope investigation has been conducted for Mr Mark and Mrs Kim Phillips-Haines at the site of a proposed residential development at Harveys Road, North Motton (TR 131560/2).

A review of the Land Information System Tasmania (LIST) website indicates that portions of the site are mapped within medium landslide hazard bands, i.e. areas of doubtful stability. As such, a landslide risk assessment is required to satisfy the ground hazard code requirements for the Interim Planning Scheme.

The investigation has been conducted to provide the following:

- A landslide risk assessment;
- Recommendations and good hillside practices to maintain or possibly lower the landslide risks;
- An assessment of the general subsurface conditions at the site and consequently assign a Site Classification in accordance with AS 2870 – 2011 "Residential Slabs and Footings";
- A review of the topographical setting and provide a Wind Classification in accordance with AS 4055 2012 "Wind Loads for Housing"; and
- The suitability of the site for disposal of domestic wastewater and the design of an onsite wastewater disposal system in accordance with AS/NZS 1547:2012 "On-site domestic wastewater management".

A basic layout plan showing the location of the proposed dwelling was provided, unreferenced. We understand the proposed dwelling will be a single storey three bedroom dwelling.

2 BACKGROUND

2.1 Geology

The MRT Digital Geological Atlas, 1:25,000 Series, indicates that the site is located on Cambrian Period basalt, with this being generally confirmed by the field data.

2.2 Landslide Hazards

Examination of the LIST Landslide Planning Map – Hazard Bands, indicates that the slopes downslope to the northwest of the proposed development area are mapped within a medium landslide hazard band. The proposed development area (including the wastewater disposal area is mapped within a low landslide hazard band.

A search of the Mineral Resourced Tasmania (MRT) database revealed that there are no known mapped landslides within the area.

3 FIELD INVESTIGATION

The field investigation was conducted on 24 July 2020 and involved the excavation of 6 test pits with a 4 tonne excavator to depths of 0.4m to 2.2m.

Insitu vane shear strength and pocket penetrometer tests were conducted in the clay layers encountered in the investigation, with samples of these soils being obtained for subsequent laboratory testing.

The results of the field and laboratory tests are shown on the test pit logs, whilst the laboratory tests are also summarised in Table 1 within Section 4.3 below.

The logs of the test pits are included in Appendix A with their locations shown in Figure 1 attached.

4 SITE CONDITIONS

4.1 Site Description

The site is an 18.6ha undeveloped lot that is currently predominantly native bush with only minor cleared areas. The proposed development area is located within the top third section of a predominantly northwest facing hill slope with an average slope of 18^o.

The proposed dwelling location has been modified by past cut to fill earthworks to create a relatively level building platform (see Plate 1 & Figure 1). Highly weathered rock was identified within the south western portion of the cut platform.

The cut batter above the proposed dwelling is approximately 3m high with a uniform slope of 26°. The cut batter was in a good condition and typically consisted of clayey silt soils overlying extremely weathered material and highly weathered rock.

The fill batter below the proposed dwelling to the north has a slope of approximately 28°. The majority of the proposed dwelling is located in the area of cut (see Figure 1).

The slopes above and below the proposed dwelling are typically subdued smooth convex slopes and do not show any obvious signs of any recent significant landslide activity. Furthermore, scattered insitu rocky outcrops were identified on the surrounding slopes (see Plate 2).

The proposed wastewater disposal field is located to the northeast of the dwelling on a gentle slope of approximately 5^o towards the north. The proposed wastewater disposal area currently has a ground surface of shrubs and small trees (see Plate 3).

No springs or seeps were observed in close proximity to the proposed building envelope or the proposed wastewater disposal field. Also, no springs or seeps were observed upslope or downslope of the proposed building envelope.

4.2 Subsurface Conditions

The test pits for the proposed dwelling varied across the site. Test Pit TP1 located within the south western portion of the proposed dwelling location (within the area of cut) encountered highly weathered rock (basalt) to the excavator refusal depth of 0.4m. Test Pit TP2 located within the south eastern portion of the of the proposed dwelling location (within the area of cut) encountered natural clayey silt to depths of 1.0m, overlying extremely weathered material to depths of 1.25m, underlain by highly weathered rock to the refusal depths of 2.2m. Test Pits TP3 and TP4 located within the northern portion of the proposed dwelling location (within the area of fill) encountered uncontrolled fill depths of 0.5m and 0.6m, overlying natural clayey silt

to depths of 1.10m and 1.05m, overlying extremely weathered material to depths of 1.25m and 1.50m, underlain by highly weathered rock to the refusal depths of 1.4m and 1.8m.

The test pits for the proposed wastewater disposal field (Test Pits TP5 and TP6) encountered clayey silt topsoil to depths of 0.15m, overlying residual clayey silt to the investigated depths of 1.5m. A photograph of Test Pit TP5 is provided as Plate 4.

The test pits did not encounter any sign of seepage over the investigated depths.

Full details of soil conditions encountered are presented on the test pit logs.

4.3 Laboratory Testing

The laboratory test results are summarised in Table 1 below:

Sample Identification	TP3 – 0.7m to 0.9m	
Liquid Limit (%)	58	
Plastic Limit (%)	33	
Plastic Index (%)	25	
Linear Shrinkage (%)	14.0	
Classification	МН	
Soil Category	Clayey Silt	

 Table 1: Summary of Laboratory Test Results

The clayey silt soil sample returned a Liquid Limit of 58%, which indicates that the clayey silt soils possess a moderate to high shrink/swell potential.

Published correlations between Plastic Index and angle of internal friction indicated that the laboratory tested soils would have peak angle of internal friction value of approximately 28° to 33°.

5 GEOLOGICAL MODEL

From a review of available reports, geological maps and information collected during the investigation a general geological model of the site has been inferred. Generally, the site comprises basalt-derived residual clayey silt soils, underlain by Cambrian Period basalt.

Groundwater was not encountered in the investigation.

6 LANDSLIDE RISK ASSESSMENT

Due to the site being partially mapped within medium landslide hazard bands, a site-specific landslide risk assessment has been carried out.

Based on the geological and geomorphological settings of the site, the following possible landslide scenarios are identified for the site.

- Deep-seated/large-scale landslide occurs on Cambrian Period basalt affecting the proposed development; and
- Shallow/small-scale landslide occurs on Cambrian Period residual basalt soils affecting the proposed development.

The qualitative likelihood, consequence and risk terms used in this report for risk to property are given in Appendix B. The risk terms are defined by a matrix that brings together different combinations of likelihood and consequence. Risk matrices help to communicate the results of risk assessment, rank risks, set priorities and develop transparent approaches to decision making. The notes attached to the tables and terms and the comments on response to risk in Appendix B are intended to help explain the risk assessment and management process.

In light of the findings of this investigation (topography, slope angles, stiff soils, shallow insitu rock and no seepages), the likelihood of small-scale failures occurring on the site affecting a proposed residential development at this site is considered UNLIKELY, whilst a larger scale failure occurring is considered RARE.

Accordingly, the likelihoods estimated for the possible landslide scenarios are summarised in Table 2 as follows.

Possible Landslide Scenarios	Indicative Annual Probability (pa)	Indicative Recurrence Interval (yrs)	Descriptor (AGS 2007c)
Deep-seated/large-scale landslide occurs on Cambrian Period basalt affecting the proposed development	10 ⁻⁵	100,000	Rare
Shallow/small-scale landslide occurs on Cambrian Period residual basalt soils affecting the proposed development	10 ⁻⁴	10,000	Unlikely

Table 2: Summary of Estimated Pre-existing Landslide Hazard

6.1 Incremental Landslide Hazards

The alterations to the site as a result of the proposed development can generally be classified into two categories:

- Disturbance to the site due to the proposed development; and
- Introduction of additional water into the ground affecting the groundwater regime.

It is considered that the proposed development would not adversely impact on the site and immediate surrounds nor significantly increase the pre-existing landslide hazard, provided that the development adheres to the principles of good hillside practice and the recommendations provided below.

The site will collect rainwater from within the site to be used domestically, and as such, only limited water will be introduced into the ground at the site and excess stormwater discharged appropriately as per our recommendation below.

6.2 Landslide Consequences

The proposed development is the element at risk for this assessment.

The landslide consequences for different scenarios are summarised in Table 3 as follows.

Table 3: Summary of Consequences for Different Landslide Scenarios

Possible Landslide Scenarios	Assessed Landslide Consequences	Descriptor (AGS 2007c)
Deep-seated/large-scale landslide occurs on Cambrian Period basalt affecting the proposed development	The landslide may significantly displace the footing system of the proposed development causing major damage	Major
Shallow/small-scale landslide occurs on Cambrian Period residual basalt soils affecting the proposed development	The landslide may displace the footing system of the proposed development causing medium damage	Medium

6.3 Landslide Risk to Property

Based on the outcomes of the landslide hazard and landslide consequence assessments detailed above. The assessed landslide risks to property are summarised in Table 4 as follows.

Table 4: Summary of Assessed Landslide Risks to Property (AGS 2007c)

Possible Landslide Scenarios	Assessed Landslide Hazards	Assessed Landslide Consequences	Qualitative Landslide Risk to Property
Deep-seated/large-scale landslide occurs on Cambrian Period basalt affecting the proposed development	Rare	Major	Low
Shallow/small-scale landslide occurs on Cambrian Period residual basalt soils affecting the proposed development	Unlikely	Medium	Low

The **acceptable** qualitative risk to property criteria suggested by AGS is **LOW**, given that the element at risk is a proposed low-rise residential development located on an existing slope.

6.4 Landslide Risk to Life

The person most at risk is considered to be a resident living in the proposed development.

The landslide risk to life for the identified person most at risk is calculated in Table 5 as follows.

Possible Landslide Scenarios	Adopted Annual Landslide Probability, P(H)	Spatial Probability of Landslide Impacting Buildings at Risk, P(S:H)	Temporal Spatial Probability of Person Most at Risk at Buildings at Risk, P(T:S)	Vulnerability of Person Most at Risk, V(D:T)	Risk to Life, R(LoL)
Deep-seated/large- scale landslide occurs on Cambrian Period basalt affecting the proposed development	10 ⁻⁵	1.0 (Spatial Probability has been	0.67	0.5 (Building suffers major damage, but is unlikely to collapse, may cause injury, but death is unlikely)	3.4 x 10 ⁻⁶
Shallow/small-scale landslide occurs on Cambrian Period residual basalt soils affecting the proposed development	10 ⁻⁴	considered in the landslide hazards)	(16hrs/day)	0.05 (Building suffers medium damage, but is highly unlikely to collapse, may cause injury, but death is highly unlikely)	3.4 x 10 ⁻⁶
Total: 6.8 x 10 ⁻⁶					

Table 5: Landslide Risk to Life for Person Most at Risk

The tolerable risk to life criteria for the person most at risk suggested by AGS is 10⁻⁵, given that the development is a new development located on an existing slope. Acceptable risks are usually considered to be one order of magnitude lower than the tolerable risks, which in this case is 10⁻⁶.

Therefore, subject to compliance with the recommendations within Section 7 of this report, the corresponding quantitively risk posed by landslide to life by the proposed development is assessed as TOLERABLE.

7 DISCUSSION AND RECOMMENDATIONS

Based on the findings of the investigation and the above landslide risk appraisal, we consider that the proposed development would not adversely impact on the site and immediate surroundings nor significantly increase its current assessed landslide risk, provided the development adheres to the principles of good hillside practice, and the recommendations below. An information sheet entitled "Some Guidelines for Hillside Construction" adapted from the Journal of the Australian Geomechanics Society, Volume 42, Number 1, dated March 2007, is presented in Appendix C.

Therefore, provided the development of the site is in accordance with good hillside practices and the recommendations within our report, we consider that a tolerable level of risk can be achieved for the development of the site in accordance with section E6.6.2 (Development on land exposed to a natural hazard) of the Hazard Management Code of the Interim Planning Scheme 2013.

An Engineering Certificate addressing the Hazard Management Code is provided in Appendix D.

We recommend that:

- The proposed dwelling is to be founded on highly weathered rock or better (see Section 9 below);
- Additional cuts and fills may be battered at slope angles no steeper than 1 vertical to 3 horizontal (1V:3H) or alternatively these should be retained;
- Additional cuts and fills greater than 1.5m in height should be reviewed by a qualified geotechnical engineer;
- All retaining walls greater than 1m in height shall be designed by a suitably qualified structural engineer;
- Adequate subsurface and surface drainage should be provided behind all retaining walls;
- A cut-off drain is required to be installed above the existing cut batter face to limit surface flows discharging over the cut batter face. We consider that the existing track located above the cut batter face can be regraded to act as the cut-off drain;
- Collected stormwater may be disposed to the area downslope and to the northeast of the proposed onsite wastewater disposal area, or alternatively to a natural drainage depression or watercourse, as appropriate; and
- All on-site wastewater is required to be disposed of in the area shown on Figure 1.

8 SITE CLASSIFICATION

Because the site is within a potential landslide risk area and has fill to depths greater than 0.4m, the site has been classified as:

CLASS P (AS 2870)

However, if footings are founded through the fill to found uniformly on the highly weathered or better rock, footings may be proportioned to a **CLASS A**.

Foundation designs in accordance with this classification are to be subject to the overriding conditions of the foundation section below.

This Classification is applicable only for ground conditions encountered at the time of this investigation. If cut or fill earthworks are carried out, then the Site Classification will need to be re-assessed, and possibly changed.

9 FOUNDATIONS

Particular attention should be paid to the design of footings as required by AS 2870 - 2011.

In addition to normal founding requirements arising from the above classification, particular conditions at this site dictate that the founding medium for all footings would be as follows:

BEDROCK (BASALT) – highly weathered or better

encountered below 0.0m to 1.5m from the existing ground surface

An allowable bearing pressure of **200kPa** is available for edge beams, strips, pads and bored piers founded as above.

Where the footings are founded on rock and deeper weathered zones are encountered, the clay should be removed and replaced with fine crushed rock or lean-mix concrete to ensure the lightly reinforced footings do not span an appreciable distance between rock outcrops.

The site classification presented assumes that the current natural drainage and infiltration conditions at the site will not be markedly affected by the proposed site development work. Care should therefore be taken to ensure that surface water is not permitted to collect adjacent to the structure and that significant changes to seasonal soil moisture equilibria do not develop as a result of service trench construction or tree root action.

Attention is drawn to Appendix B of AS 2870 and CSIRO Building Technical File BTF18 "Foundation Maintenance and Footing Performance: A Homeowner's Guide" as a guide to maintenance requirements for the proposed structure.

Although the test pit data provides an indication of subsurface conditions at the site, variations in soil conditions may occur in areas of the site not specifically covered by the field investigation. The base of all footing or beam excavations should therefore be inspected to ensure that the founding medium meets the requirements referenced herein with respect to type and strength of founding material.

10 WIND CLASSIFICATION

After allowing due consideration of the region, terrain, shielding and topography, the site has been classified as follows:

REGION	TERRAIN CATEGORY SHIELDING		TOPOGRAPHY
A	TC3	NS	T4

WIND CLASSIFICATION N3 (AS 4055)

11 ADDITIONAL WORK

As the site is partially within a medium landslide risk band, Council may require that the plans of the proposed development be reviewed by a qualified geotechnical practitioner to ensure that the proposed plans are in keeping with the general recommendations of the report.

In addition, it may be a requirement of the building permit that Geoton carry out site inspections during the earthworks and foundations excavation; and at the end of construction to provide a final geotechnical certificate.

12 EFFLUENT DISPOSAL

The AS/NZS 1547:2012 provides a guide to typical wastewater flow allowances under a range of circumstances. The standard recommends a typical wastewater flow of 120L/person/day for

households on tank water. As the proposed development is to be a dwelling with three bedrooms, a population equivalent of 5 has been adopted. As such, the wastewater daily flow will be **600L/day**.

12.1 Permeability of Soil and Soil Category

The soil has been classified as follows:

- Texture Clayey Silt/Loam (Table E1 from AS1547-2012);
- Structure Moderately Structured (Table E4 from AS/NZS1547-2012); and
- Category 4 (Table E1 from AS/NZS1547:2012).

The permeability (K_{sat}) at the site was measured at 0.6m/day. For moderately structured Category 4 soils the indicative permeability from AS/NZS1547 Table 5.1 is 0.5-1.5m/day. Therefore, the permeability is within the range for moderately structured Category 4 soils.

• Adopted Permeability – 0.6m/day.

12.2 Disposal and Treatment Method

This site assessment indicates that the site is suitable for the disposal of domestic effluent by way of a septic tank, which is required to have a minimum capacity of **3000L**, and absorption trenches.

The soils within the proposed effluent disposal area are assessed as having sufficient depth and clay content to provide an adequate attenuation period for the breakdown of pathogens within the treated effluent.

12.3 Design Loading Rate

The adopted design loading rate for the absorption trenches has been set at 15mm/day as outlined in AS/NZS 1547:2012 Table L1.

12.4 Absorption Trench System

Guidelines for the design of the trench systems are outlined in AS/NZS 1547:2012 Appendix L. The method of determining the dimensions for the trenches is outlined in AS/NZS 1547:2012 Section L4 and is as follows:

Where L = Length in metres

Q = Design daily flow in L/day

DLR = Design Loading Rate in mm/day

W = Trench width in metres (set at 1.0m)

As the DLR value has been set at 15mm/day and the design daily flow (Q) has been set at 600L/day, when the parameters are inserted in the above equation the trench dimensions required are as follows:

- Trench length = 40m (**2 x 20m trenches**)
- Trench width = 1.0m
- Trench depth = 0.45m

The trenches are to be installed along the contours and below each other.

The disposal field for the above scenario would need to be a minimum of 24m long and 9m wide, due to the following conditions:

- A 2m buffer is required around the outside of the disposal field; and
- A downslope separation of 3m (minimum) must be left between trenches.

This would give a disposal area of approximately 216m². These dimensions may be modified to suit the client's needs provided that the total length remains and the spacing between and around the trenches are adhered to.

It is noted that AS/NZS 1547:2012 recommends that trench lengths be limited to about 20m, however a longer trench is possible provided the installer can guarantee a level base over the proposed length.

There is adequate reserve area of 216m² if required.

The trenches are to be located in the area shown on the site plan. A distribution box is to be installed to ensure even distribution of effluent to the two trenches.

The trenches are to be constructed as per the cross sections located on Figure WW-02 attached. Additional design details for the disposal field are as follows:

• A cut-off drain is required to be installed around the system to limit surface run off impacting on the distribution area. We consider the existing driveway can be utilised to act as the cut-off drain.

12.5 Setbacks

The minimum separation distance between the disposal area and downslope features is based on Appendix R from AS/NZS 1547:2012 "Recommended Setback Distances for Land Application Systems". As per Table R1 from AS/NZS 1547:2012 a setback of 100m is required from downslope watercourse and sensitive features, a setback of 30m is required from downslope property boundaries and a setback of 3m is required for buildings and property boundaries located cross-slope or up-slope.

12.6 Wastewater Recommendations

It is recommended that the following actions are undertaken in looking after your system:

- Minimise domestic water use;
- Minimise the use of non-biodegradable detergents;
- Minimise the use of detergents containing phosphorous (e.g. Calgon or similar);
- Avoid discharging polluting chemicals into wastewater systems; and
- Monitor quality of groundwater.

References:

Australian Geomechanics Society (2007) – Practice note guidelines for landslide risk management 2007, Australian Geomechanics Journal, Vol 42, No. 1

AS 1726 - 2017 Geotechnical Site Investigation

AS 2870 - 2011 Residential Slabs and Footings

AS 4055 - 2012 Wind Loads for Housing

AS/NZS 1547 - 2012 On-site domestic wastewater management

GEOTON Pty Ltd Geotechnical Consultants - Limitations of report

These notes have been prepared to assist in the interpretation and understanding of the limitations of this report.

Project specific criteria

The report has been developed on the basis of unique project specific requirements as understood by Geoton and applies only to the site investigated. Project criteria are typically identified in the Client brief and the associated proposal prepared by Geoton and may include risk factors arising from limitations on scope imposed by the Client. The report should not be used without further consultation if significant changes to the project occur. No responsibility for problems that might occur due to changed factors will be accepted without consultation.

Subsurface variations with time

Because a report is based on conditions which existed at the time of subsurface exploration, decisions should not be based on a report whose adequacy may have been affected by time. For example, water levels can vary with time, fill may be placed on a site and pollutants may migrate with time. In the event of significant delays in the commencement of a project, further advice should be sought.

Interpretation of factual data

Site assessment identifies actual subsurface conditions only at those points where samples are taken and at the time they are taken. All available data is interpreted by professionals to provide an opinion about overall site conditions, their likely impact on the proposed development and recommended actions. Actual conditions may differ from those inferred to exist, as it is virtually impossible to provide a definitive subsurface profile which includes all the possible variabilities inherent in soil and rock masses.

Report Recommendations

The report is based on the assumption that the site conditions as revealed through selective point sampling are indicative of actual conditions throughout an area. This assumption cannot be substantiated until earthworks and/or foundation construction is almost complete and therefore the report recommendations can only be regarded as preliminary. Where variations in conditions are encountered, further advice should be sought.

Specific purposes

This report should not be applied to any project other than that originally specified at the time the report was issued.

Interpretation by others

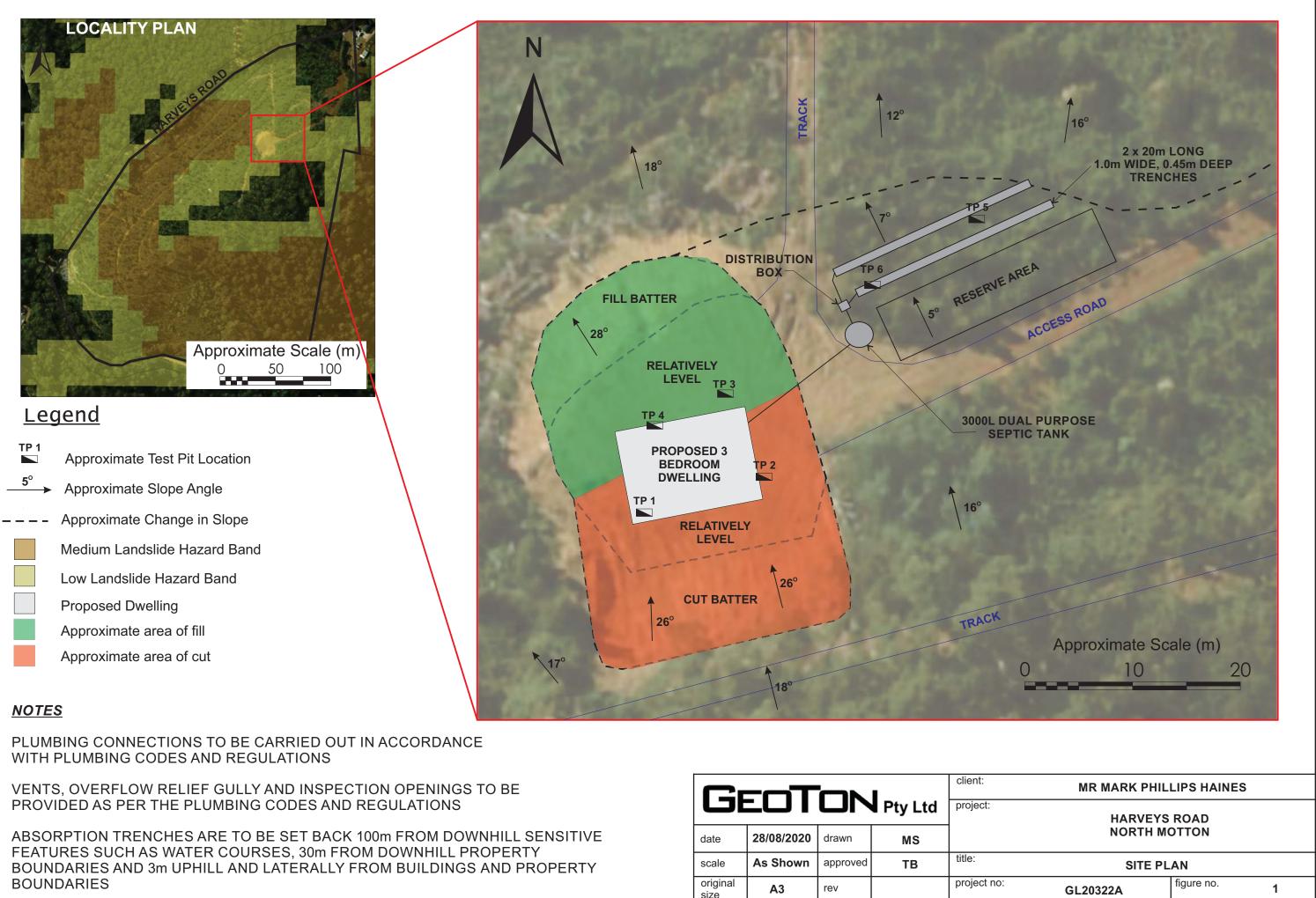
Geoton will not be responsible for interpretations of site data or the report findings by others involved in the design and construction process. Where any confusion exists, clarification should be sought from Geoton.

Report integrity

The report as a whole presents the findings of the site assessment and the report should not be copied in part or altered in any way.

Geoenvironmental issues

This report does not cover issues of site contamination unless specifically required to do so by the client. In the absence of such a request, Geoton take no responsibility for such issues.



WITH PLUMBING CODES AND REGULATIONS

PROVIDED AS PER THE PLUMBING CODES AND REGULATIONS

Ge	client: project:			
date	28/08/2020	drawn	MS	
scale	As Shown	approved	ТВ	title:
original size	A3	rev		project no:

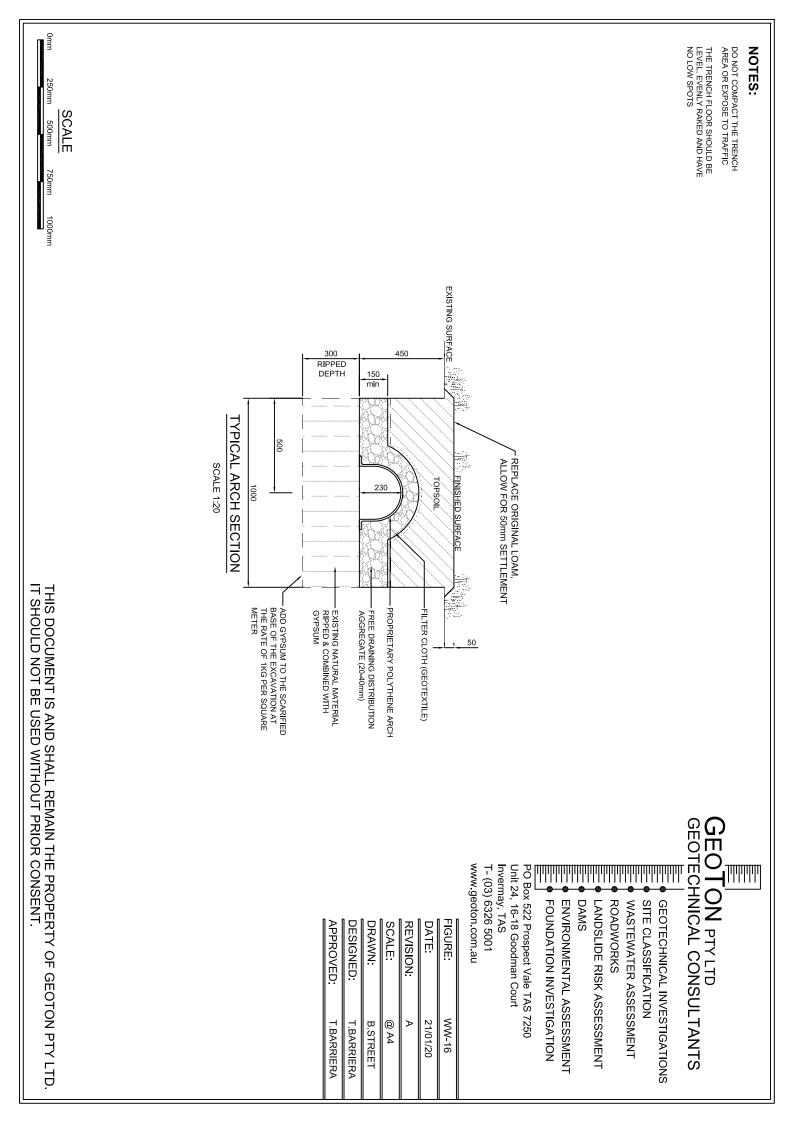




PLATE 1 - Looking southwest at the proposed building envelope and the cut batter above the site



PLATE 2 - Looking east at insitu rock outcrops located below the proposed dwelling

				client:	client: MR MARK PHILLIPS HAINES					
GF			Pty Ltd	project:	HARVEY ROAD					
title:	РНОТС	DGRAPH			NORTH MOT	TON				
date:	24/07/2020	original size	A4	project no:	GL20322A	figure no. PLATES 1 & 2				



PLATE 3 - Looking northeast at the proposed wastewater disposal field



PLATE 4 - Test Pit TP5 (located within the proposed wastewater field)

Cr			1	client: MR MARK PHILLIPS HAINES					
GF			Pty Ltd	project:	roject: HARVEY ROAD				
title:	РНОТ	DGRAPH			NORTH MOT	TON			
date:	24/07/2020	original size	A4	project no:	GL20322A	figure no. PLATES 3 & 4			



Excavation Logs

ENGINEERING EXCAVATION LOG

Geotechnical Engineers PO Box 522 Prospect TAS 7250

Unit 24, 16-18 Goodman Court, Invermay Tas

Tel (03) 6326 5001

Test Pit no. TP1 Sheet no. 1 of 1 Job no. GL20322A

		ent	-		Jon Mark Phillip	p Haine	s					Date :	24/7/20	
		-	ct :		otechnical	-						Logged By :	MS	
					veys Road	l, North	Mott							
E	xc	av	ator:	4 To	onne			Buc	0			RL Surface :		
┢	Т							Len	th: 1.5m Northing:	Ę		Datum :		П
Mathod		Support	Penetration	Water	Notes Samples Tests	Depth (m)	Graphic log	Classification Symbol	Material Description	Moisture condition	Consistency density, index	Structure, observ		
						- - -			BEDROCK (Basalt) - Highly Weathered, low strength, fine grained, light brown	D/IVI	VD			
	1	z				0.25								
						- 0.50 - 0.75 - 0.75 - 1.00 - 1.25 - 1.50 - 1.75 			Test Pit TP1 terminated at 0.4m Excavator refusal on moderately weathere rock (basalt)					<u>. . </u>
						2.50								

ENGINEERING EXCAVATION LOG

Geotechnical Engineers PO Box 522 Prospect TAS 7250

Unit 24, 16-18 Goodman Court, Invermay Tas

Tel (03) 6326 5001

Test Pit no. TP2 Sheet no. 1 of 1 Job no. GL20322A

	ient	,	26 50 Mr N	/ark Phillip	o Haine	s					Date : 24/	7/20
	oje			technical	•						Logged By : N	ΛS
				/eys Road	l, North	Mott						
Ex	(ca)	/ator:	4 Tc	onne		Bucket: 0.6m Easting:					RL Surface :	
			1				Len	th: 1.5m Northing:	Datum :			
Method	Support	Penetration	Water	Notes Samples Tests	Depth (m)	Graphic log	Classification Symbol		Moisture condition	Consistency density, index	Structure, addit observation	
					0.25 0.50 0.75 1.00 1.25 1.50 1.75 2.00		ML/ MH	Clayey SIL1 - medium/high plasticity, light brown/cream, with gravel (highly weathered basalt) With a weak rock fabric EXTREMELY WEATHERED MATERIAL - Clayey SILT soil properties, low plasticity, light brown & black, with sand/gravel, distinct rock fabric BEDROCK (Basalt) - Highly Weathered, low strength, fine grained, light brown	M D/M	VST	V-110kPa V-refusal pp>400kPa pp>400kPa pp>400kPa	
					2.50							

ENGINEERING EXCAVATION LOG

Geotechnical Engineers PO Box 522 Prospect TAS 7250

Unit 24, 16-18 Goodman Court, Invermay Tas

Tel (03) 6326 5001

Test Pit no. TP3 Sheet no. 1 of 1 Job no. GL20322A

CI	ient	t :	Mr I	Mark Philli	p Haine	s					Date : 24/7/20
Pr	oje	ct :	Geo	otechnical	Investig	atior	ı				Logged By : MS
Lo	cat	ion :	Har	veys Road	l, North	Mott	on				
Ex	cav	vator:	4 To	onne			Buc	6	RL Surface :		
_	_			I	1		Len	th: 1.5m Northing:	-		Datum :
Method	Support	Penetration	Water	Notes Samples Tests	Depth (m)	Graphic log	Classification Symbol	Material Description	Moisture condition	Consistency density, index	
					- - - - - - - - - - - - - - - - - - -			FILL - Mixture of Clayey SILT & Silty CLA medium/high plasticity, brown, yellow and light brown, with a trace of organics	r M	F	FILL - - - - - - - - - - - - - - - -
ш	z			D	- 0.75 - - - - - - - - - - - - - - - - - - -		ML/ MH	Clayey SILT - medium/high plasticity, light orange/cream, with gravel (highly weathered basalt)	М	VSt	NATURAL Liquid Limit = 58% Plastic Limit = 33% Plasticity Index = 25% Linear Shrinkage = 14% V-refusal
					- - - - - - - - - - - - - - - - - - -			EXTREMELY WEATHERED MATERIAL - Clayey SILT soil properties, low plasticity, light brown & black, with sand/gravel, distinct rock fabric BEDROCK (Basalt) - Highly Weathered, low strength, fine grained, light brown		VD	- - - - - - - - - - - - - - - - - - -
					2.25			Test Pit TP3 terminated at 1.8m Excavator refusal on moderately weathere rock (basalt)	c		

ENGINEERING EXCAVATION LOG

Geotechnical Engineers PO Box 522 Prospect TAS 7250

Unit 24, 16-18 Goodman Court, Invermay Tas

Tel (03) 6326 5001

Test Pit no. TP4 Sheet no. 1 of 1 Job no. GL20322A

Г	Cli	-	-	Mr N	Mark Phillip	o Haine	s					Date :	24/7/20
	Pro	oje	ct :		technical			ו				Logged By :	MS
	Lo	cat	ion :	Har	veys Road	l, North	Mott	ton					
	Ex	ca	vator	4 To	onne			Buc	Ŭ			RL Surface :	
Ļ								Len	th: 1.5m Northing:	_	1	Datum :	
	Method	Support	Penetration	Water	Notes Samples Tests	Depth (m)	Graphic log	Classification Symbol	Material Description	Moisture condition	Consistency density, index		
						- - - - - - - - - - - - - - - - - - -		KAL 7	FILL - Mixture of Clayey SILT & Silty CLAY medium/high plasticity, brown, yellow and light brown, with a trace of organics				
	Е	z				0.75		ML/ MH	Clayey SILT - medium/high plasticity, light orange, with gravel (highly weathered basalt)	М	VSt	NATURAL V-100kPa	
						1.00			Becoming yellow/brown Weak rock fabric				
						- - 1.25			EXTREMELY WEATHERED MATERIAL - Clayey SILT soil properties, low plasticity, light brown & black, with sand/gravel, distinct rock fabric BEDROCK (Basalt) - Highly Weathered, low strength, fine grained, light brown	M D/M	D VD		• • •
						-			Test Pit TP4 terminated at 1.4m				
						 			Excavator refusal on moderately weathered rock (basalt)				- - -
						<u> 1.75 </u>							
						2.00							
						2.25							

ENGINEERING EXCAVATION LOG

Geotechnical Engineers PO Box 522 Prospect TAS 7250

Unit 24, 16-18 Goodman Court, Invermay Tas

Tel (03) 6326 5001

Test Pit no. TP5 Sheet no. 1 of 1 Job no. GL20322A

Г	Clie	ent	:	Mr N	Mark Phillip	o Haine	s					Date : 24/7/2	20
			ct :		technical			ı				Logged By : MS	
	Loc	cat	ion :		veys Road	-							
	Exe	ca∖	/ator:	4 To	onne			Buc	ket: 0.6m Easting:			RL Surface :	
								Length: 1.5m Northing:			Datum :		
	Method	Support	Penetration	Water	Notes Samples Tests	Depth (m)	Graphic log	Classification Symbol		Moisture condition	Consistency density, index	Structure, additiona observations	al
						_		ML	TOPSOIL - Clayey SILT, low plasticity, brown, trace organic matter	М	F		-
						0.25		ML/ MH	Clayey SILT - medium/high plasticity, red/brown	М	St		-
						-							-
						0.50							_
						- 0.75							-
	ш	Z											-
						_ 							-
						-							-
						1.25							_
						-							-
	-			_		1.50			Test Pit TP5 terminated at 1.5m	_			
						F							1
						L							Ŀ
						1.75							_
						_							1
													Ŀ
						2.00							_
						F							1
						- 							╢
						2.25							
						╞							┨
						2.50							-

ENGINEERING EXCAVATION LOG

Geotechnical Engineers PO Box 522 Prospect TAS 7250

Unit 24, 16-18 Goodman Court, Invermay Tas

Tel (03) 6326 5001

Test Pit no. TP6 Sheet no. 1 of 1 Job no. GL20322A

(ent		Mr N	/lark Phillij	p Haine	s					Date : 24/7/20	
	Pro	jec	t :		technical			۱				Logged By : MS	
					veys Road	l, North	Mot						
	Ξxc	av	ator:	4 To	onne			Bucket: 0.6m Easting:			RL Surface :		
┢				1		1		Len	th: 1.5m Northing:			Datum :	
	Method	Support	Penetration	Water	Notes Samples Tests	Depth (m)	Graphic log	Classification Symbol	Material Description	Moisture condition	Consistency density, index	Structure, additional observations	
						-		ML	TOPSOIL - Clayey SILT, low plasticity, brown, trace organic matter	М	F	-	
						0.25		ML/ MH	Clayey SILT - medium/high plasticity, red/brown, trace cobbles	М	St	-	
						-						-	
						0.50						-	
												-	
	ш	z											
						_ 						-	
						-						-	
						1.25						-	
						- -						-	
∥						1.50			Test Pit TP6 terminated at 1.5m				
												-	
						L						-	
						1.75						_	
						F							
						_						-	
						2.00						_	
						F							
						2.25							
						2.20							
						-						-	
						2.50							

Investigation Log Explanation Sheet

Method – Borehole

TERM	Description					
AS	Auger Screwing*					
AD	Auger Drilling*					
RR	Roller / Tricone					
W	Washbore					
СТ	Cable Tool					
HA	Hand Auger					
DT	Diatube					
В	Blank Bit					
V	V Bit					
Т	TC Bit					

* Bit shown by suffix e.g. ADT

Method – Excavation

TERM	Description				
Ν	Natural exposure				
х	Existing excavation				
н	Backhoe bucket				
В	Bulldozer blade				
R	Ripper				
E	Excavator				

Support

TERM	Description						
М	Mud						
N	Nil						
С	Casing						
S	Shoring						

Penetration

1	2	3	4	
				No resistance ranging to refusal

Water

Symbol	Description
	Water inflow
	Water outflow
	17/3/08 water on date shown

Notes, samples, tests

TERM	Description	
U ₅₀	Undisturbed sample 50 mm diameter	
U ₆₃	Undisturbed sample 63 mm diameter	
D	Disturbed sample	
Ν	Standard Penetration Test (SPT)	
N*	SPT – sample recovered	
Nc	SPT with solid cone	
V	Vane Shear	
PP	Pocket Penetrometer	
Р	Pressumeter	
Bs	Bulk sample	
Е	Environmental Sample	
R	Refusal	
DCP	Dynamic Cone Penetrometer (blows/100mm)	

Classification symbols and soil description

Based on unified classification system

Moisture

TERM	Description
D	Dry
М	Moist
W	Wet
WP	Plastic Limit
WL	Liquid Limit

Consistency/Density index

TERM	Description
VS	very soft
S	soft
F	firm
St	stiff
VSt	very stiff
н	hard
Fb	friable
VL	very loose
L	loose
MD	medium dense
D	dense
VD	Very dense

Soil Description Explanation Sheet(1of 2)

DEFINITION:

In engineering terms soil includes every type of uncemented or partially cemented inorganic or organic material found in the ground. In practice, if the material can be remoulded or disintegrated by hand in its field condition or in water it is described as a soil. Other materials are described using rock description terms.

CLASSIFICATION SYMBOL & SOIL NAME

Soils are described in accordance with the Unified Classification System (UCS) as shown in the table on Sheet 2.

PARTICLE SIZE DESCRIPTIVE TERMS

NAME	SUBDIVISION	SIZE
Boulders		>200 mm
Cobbles		63 mm to 200 mm
Gravel	coarse	20 mm to 63 mm
	medium	6 mm to 20 mm
	fine	2.36 mm to 6 mm
Sand	coarse	600 µm to 2.36 mm
	medium	200 µm to 600 µm
	Fine	75 µm to 200 µm

MOISTURE CONDITION

- Dry Looks and feels dry. Cohesive and cemented soils are hard, friable or powdery. Uncemented granular soils run freely through hands.
- Moist Soil feels cool and darkened in colour. Cohesive soils can be moulded. Granular soils tend to cohere.
- Wet As for moist but with free water forming on hands when handled.

CONSISTENCY OF COHESIVE SOILS

TERM	UNDRAINED STRENGTH s _u (kPa)	FIELD GUIDE
Very Soft	<12	A finger can be pushed well into the soil with little effort.
Soft	12 - 25	A finger can be pushed into the soil to about 25mm depth.
Firm	25 - 50	The soil can be indented about 5mm with the thumb, but not penetrated.
Stiff	50 - 100	The surface of the soil can be indented with the thumb, but not penetrated.
Very Stiff	100 - 200	The surface of the soil can be marked, but not indented with thumb pressure.
Hard	>200	The surface of the soil can be marked only with the thumbnail.
Friable	_	Crumbles or powders when scraped by thumbnail.

DENSITY OF GRANULAR SOILS

TERM	DENSITY INDEX (%)
Very loose	Less than 15
Loose	15 - 35
Medium Dense	35 - 65
Dense	65 - 85
Very Dense	Greater than 85

MINOR COMPONENTS

	r	
TERM	ASSESSMENT GUIDE	PROPORTION OF MINOR COMPONENT IN:
Trace of	Presence just detectable by feel or eye, but soil	Coarse grained soils: <5%
	properties little or no different to general properties of primary component.	Fine grained soils: <15%
With some	Presence easily detected by feel or eye, soil properties	Coarse grained soils: 5 - 12%
	little different to general properties of primary component.	Fine grained soils: 15 - 30%

SOIL STRUCTURE

ZONING		CEMENTING	
Layers	Continuous across exposure or sample.	Weakly cemented	Easily broken up by hand in air or water.
Lenses	Discontinuous layers of lenticular shape.	Moderately cemented	Effort is required to break up the soil by hand in air or water.
Pockets	Irregular inclusions of different material.		

GEOLOGICAL ORIGIN

WEATHERED IN PLACE SOILS

Extremely weathered material	Structure and fabric of parent rock visible.
Residual soil	Structure and fabric of parent rock not
	visible.

TRANSPORTED SOILS

Aeolian soil	Deposited by wind.
Alluvial soil	Deposited by streams and rivers.
Colluvial soil	Deposited on slopes (transported
	downslope by gravity).
Fill	Man made deposit. Fill may be significantly
	more variable between tested locations
	than naturally occurring soils
Lacustrine soil	Deposited by lakes.
Marine soil	Deposited in ocean basins, bays, beaches and estuaries.

Soil Description Explanation Sheet (2 of 2)

SOIL CLASSIFICATION INCLUDING IDENTIFICATION AND DESCRIPTION

		CATION PROC		fract	ions on estimated mass	5)	USC	PRIMARY NAME
COARSE GRAINED SOILS More than 50% of materials less than 63 mm is larger than 0.075 mm	(A 0.075 mm particle is about the smallest particle visible to the naked eye)	GRAVELS More than half of coarse fraction is larger than 2.0 mm	CLEAN GRAVELS (Little or no fines)	Wide range in grain size and substantial amounts of all intermediate particle sizes.			GW	GRAVEL
			CLE GRA (Li or no	Predominantly one size or a range of sizes with more intermediate sizes missing.			GP	GRAVEL
			LS VES able nt s)	Non-plastic fines (for identification procedures see ML below)			GM	SILTY GRAVEL
			GRAVELS WITH FINES (Appreciable amount of fines)	Plastic fines (for identification procedures see CL below)			GC	CLAYEY GRAVEL
		alf of laller	CLEAN SANDS (Little or no fines)	Wide range in grain sizes and substantial amounts of all intermediate sizes missing			SW	SAND
CO/ S0 ¹ S1		SANDS More than half of coarse fraction is smaller than 2.0 mm	CL SAI (L or no	Predominantly one size or a range of sizes with some intermediate sizes missing.			SP	SAND
More th			SANDS WITH FINES (Appreciable amount of fines)	Non-plastic fines (for identification procedures see ML below).			SM	SILTY SAND
				Plastic fines (for identification procedures see CL below).			SC	CLAYEY SAND
			IDENTIFICATIO	N PR	OCEDURES ON FRAC	CTIONS < 0.2 mm.		
less an		SILTS & CLAYS Liquid limit less than 50	DRY STRENGT	Н	DILATANCY	TOUGHNESS		
GRAINED SOILS 1 50% of Material I mm is smaller the 0.075 mm			None to Low		Quick to slow	None	ML	SILT
ED S mal		S & quic šš th	Medium to High		None	Medium	CL	CLAY
RAINE 0% of m is s 075 r		SILT Li les	Low to medium		Slow to very slow	Low	OL	ORGANIC SILT
O T C T C T C T C T C T C T C T C T C T		SILTS & CLAYS Liquid limit greater than 50	Low to medium		Slow to very slow	Low to medium	MH	SILT
FINE GRAINED SOILS More than 50% of Material less than 63 mm is smaller than 0.075 mm			High		None	High	СН	CLAY
Mc t			Medium to High None		Low to medium	OH	ORGANIC CLAY	
HIGHLY ORGANIC SOILS Readily identified by colour, odour, spongy feel and frequently by fibrous texture.					Pt	PEAT		
 Low plast 	icity	– Liquid Limit W	L less than 35%.	Mec	lium plasticity – WL bet	ween 35% and 50%.		

COMMON DEFECTS IN SOIL

TERM	DEFINITION	DIAGRAM	TERM	DEFINITION	DIAGRAM
PARTING	A surface or crack across which the soil has little or no tensile strength. Parallel or sub parallel to layering (eg bedding). May be open or closed.		SOFTENED ZONE	A zone in clayey soil, usually adjacent to a defect in which the soil has a higher moisture content than elsewhere.	METON AND SHOT
JOINT	A surface or crack across which the soil has little or no tensile strength but which is not parallel or sub parallel to layering. May be open or closed. The term 'fissure' may be used for irregular joints <0.2 m in length.		TUBE	Tubular cavity. May occur singly or as one of a large number of separate or inter-connected tubes. Walls often coated with clay or strengthened by denser packing of grains. May contain organic matter	
SHEARED ZONE	Zone in clayey soil with roughly parallel near planar, curved or undulating boundaries containing closely spaced, smooth or slickensided, curved intersecting joints which divide the mass into lenticular or wedge shaped blocks.		TUBE CAST	Roughly cylindrical elongated body of soil different from the soil mass in which it occurs. In some cases the soil which makes up the tube cast is cemented.	
SHEARED SURFACE	A near planar curved or undulating, smooth, polished or slickensided surface in clayey soil. The polished or slickensided surface indicates that movement (in many cases very little) has occurred along the defect.		INFILLED SEAM	Sheet or wall like body of soil substance or mass with roughly planar to irregular near parallel boundaries which cuts through a soil mass. Formed by infilling of open joints.	

Rock Description Explanation Sheet (1 of 2)

DEFINITIO	NS:	Rock substance, defect mass are defined	l as follows:							
Rock Subst	ance	In engineering terms rock substance is any nat be disintegrated or remoulded by hand in air or homogenous material, may be isotropic or anis	water. Other m							
Defect		Discontinuity or break in the continuity of a substance or substances.								
Mass		Any body of material which is not effectively ho		can consist o	of two or more sub	stances without defects, or				
SUBSTAN	CF DF	one or more substances with one or more defe SCRIPTIVE TERMS:		SUBSTA	NCE STRENG	TH TERMS				
ROCK NAME		Simple rock names are used rather than precise geological classification	Term	Abbrev- iation	Point Load Index, Is50	Field Guide				
PARTICLE SIZE		Grain size terms for sandstone are:	Very Low	VL	(MPa) Less than 0.1	Material crumbles under				
Coarse grained		Mainly 0.6mm to 2mm			0.1	firm blows with sharp end of pick; can be peeled				
Medium grained		Mainly 0.2mm to 0.6mm				with a knife; pieces up to 30mm thick can be				
Fine grained		Mainly 0.06mm(just visible) to 0.2mm				broken by finger pressure				
FABRIC		Terms for layering of penetrative fabric (eg, bedding, cleavage etc.) are:	Low	L	0.1 to 0.3	10 Easily scored with a knife; indentations 1mm to 3mm show with firm bows of a pick point; has a dull sound under hammer. Pieces of core				
Massive		No layering or penetrative fabric								
Indistinct		Layering or fabric just visible. Little effect on properties								
Distinct		Layering or fabric is easily visible. Rock breaks more easily parallel to layering of fabric				150mm long by 50mm diameter may be broken by hand. Sharp edges of core may be friable and break during handling.				
		N OF WEATHERING PRODUCTS								
Term At Residual Soil	obreviati RS	on Definition Soil derived from the weathering of rock; the mass structure and substance fabric are no longer evident; there is a large change in volume but the soil has not been significantly transported.	Medium	М	0.3 to 1.0	Readily scored with a knife; a piece of core 150mm long by 50mm diameter can be broken by hand with difficulty.				
Extremely Weathered Material	XW	Material is weathered to such an extent that has soil properties, ie, it either disintegrates of can be remoulded in water. Original rock fab still visible.	or	н	1 to 3	A piece of core 150mm long by 50mm can not be broken by hand but can be broken by a pick with single firm blow; rock				
Highly Weathered Rock	HW	Rock strength is changed by weathering. The whole of the rock substance is discoloured, usually by iron staining or bleaching to the extent that the colour of the original rock is ne recognisable. Some minerals are decompose to clay minerals. Porosity may be increased leaching or may be decreased due to the deposition of minerals in pores.	ot ed Very Higł	י VH	3 to 10	rings under hammer. Hand specimen breaks after more than one blow of a pick; rock rings under hammer.				
Moderately Weathered Rock	MW	The whole of the rock substance is discoloured, usually by iron staining or bleaching, to the extent that the colour of the fresh rock is no longer recognisable.	Extremel High	y EH	More than 10	Specimen requires many blows with geological pic to break; rock rings unde hammer.				
Slightly Weathered Rock	eathered extent that partial staining or partial		ie	Rock Subs	stance Strength:					
Fresh Rock	FR	Rock substance unaffected by weathering	stre	 In anisotropic rocks the field guide to strength applies to the strength perpendicular to the anisotropy. High strength anisotropic rocks may break readily parallel to the planar 						
Notes on Wea	•			sotropic roc sotropy.	na may break read	ny paraner lo lite pianar				
the ran SW. Fo HW and	ge of so or projects d MW or distinctio	ts the term "Distinctly Weathered" (DW) to cover ubstance weathering conditions between XW and s where it is not practical to delineate between it is judged that there is no advantage in making n. DW may be used with the definition given in	stre	ength term d guide ther	 While the term is 	sed as a rock substance used in AS1726-1993, the that materials in that strength ms.				
 Where physical and chemical changes were caused by hot gasses and liquids associated with igneous rocks, the term "altered" may be substituted for "weathering" to give the abbreviations XA, HA, MA, SA and DA. 			s ani ay typ A, ma	3. The unconfined compressive strength for isotropic rocks (and anisotropic rocks which fall across the planar anisotropy) is typically 10 to 25 times the point load index (Is50). The ratio may vary for different rock types. Lower strength rocks often have lower ratios than higher strength rocks.						

Rock Description Explanation Sheet (2 of 2)

ROCK MAS	DEFECTS IN SSES	Diagram	Map Symbol	Graphic Log (Note 1)	DEFECT SHAPE	TERMS		
Term	Definition			ζ ,	Planar	The defect does not vary in orientation.		
Parting	A surface or crack across which the rock has little or no tensile		20	121	Curved	The defect has a gradual change in orientation.		
	strength. Parallel or sub parallel to layering (eg bedding) or a planar anisotropy in the rock substance (eg cleavage). May be open or closed.	/	20 1	Iding (Note 2)	Undulating	The defect has a wavy surface.		
					Stepped	The defect has one or more well defined steps.		
Joint	A surface or crack across which the rock has little or no tensile		6 0	KE!	Irregular	The defect has many sharp changes of orientation.		
	strength, but which is not parallel to layering or planar anisotropy in the rock substance. May be open or closed.		×	(Note 2)	partl	assessment of defect shape is y influenced by the scale of the ervation.		
					ROUGHNESS	TERMS		
Sheared Zone	Zone of rock substance with roughly parallel near planar, curved or undulating boundaries cut by closely spaced joints, sheared surfaces or other defects. Some of the defects are usually curved and intersect to divide the mass into lenticular or	iiita.	25	14-1	Slickensided	Grooved or striated surface, usually polished.		
Note 3)		N. S.	30	1 12.0	Polished	Shiny smooth surface.		
					Smooth	Smooth to touch. Few or no surface irregularities.		
	wedge shaped blocks.				Rough	Many small surface		
Sheared Surface Note 3)	A near planar, curved or undulating surface which is usually smooth, polished or slickensided.		40			irregularities (amplitude generally less than 1mm). Feels like fine to coarse sand paper.		
Crushed Seam	Seam with roughly parallel almost planar boundaries,				Very Rough	Many large surface irregularities (amplitude generally more than 1mm). Feels like, or coarser than ver coarse sand paper.		
Note 3)	amost plana boundaries, composed of disoriented, usually angular fragments of the host rock substance which may be more weathered than the host rock. The seam has soil properties.	(0) (5)	50 	A.	COATING TEF Clean	ATING TERMS		
					Stained	No visible coating but surfaces are discoloured.		
Infilled Seam	Seam of soil substance usually with distinct roughly parallel	1			Veneer	A visible coating of soil or mineral, too thin to measure;		
	boundaries formed by the migration of the soil into an open cavity or joint, infilled seams less than 1mm thick may be described as a veneer coating on the joint surface.		Real Provide P	65	Coating	may be patchy. A visible coating up to 1mm thick. Thicker soil material is usually described using appropriate defect terms (eg, infilled seam). Thicker rock strength material is usually described as a vein.		
					BLOCK SHAP	E TERMS		
Extremely Veathered Seam	Seam of soil substance, often with gradational boundaries. Formed by weathering of the	1000000	32	EL A	Blocky	Approximately equidimensional.		
	rock substance in place.	Seam	THE	94 ····	Tabular	Thickness much less than length or width.		
					Columnar	Height much greater than cross section.		
Notes on D 1.	efects: Usually borehole logs show the true dip of o	defects and face s	ketches and s	ections the annare	ent dip.			
1. 2.	Partings and joints are not usually shown o							
۷.	ge and jenne all not addaily chowing							

Appendix B

Qualitative Terminology for Use in Assessing Risk to Property

QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		ility Implied Indicative Landslide		Description	Descriptor	Level
Indicative Value	Notional Boundary	Recurrenc	e Interval			
10 ⁻¹	5x10-2	10 years		The event is expected to occur over the design life.	ALMOST CERTAIN	A
10-2	5x10-2	100 years	20 years	The event will probably occur under adverse conditions over the design life.	LIKELY	В
10-3	5x10-3	1000 years	200 years	The event could occur under adverse conditions over the design life.	POSSIBLE	С
10-4	5x10-4	10,000 years	2000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10-5	5x10-5	100,000 years	20,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10-6	5,10-0	1,000,000 years	200,000 years	The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

Note: (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not vice versa.

QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	1000/	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%	100%	Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	40% 10%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works. Could cause at least one adjacent property minor consequence damage.	MEDIUM	3
5%		Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works.	MINOR	4
0.5%	1%	Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

Notes: (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved value of the unaffected property which includes the land plus the unaffected structures.

(3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilization works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.

(4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not vice versa

Geoton Pty Ltd (adapted from Australian Geomechanics Vol 42 No 1 March 2007)

QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY

LIKELIH	OOD	CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)					
	Indicative Value of Approximate Annual Probability	1: CATASTROPHIC 200%	2: MAJOR 60%	3: MEDIUM 20%	4: MINOR 5%	5: INSIGNIFICANT 0.5%	
A – ALMOST CERTAIN	10 ⁻¹	VH	VH	VH	Н	M or L (5)	
B - LIKELY	10 ⁻²	VH	VH	н	М	L	
C - POSSIBLE	10 ⁻³	VH	н	М	М	VL	
D - UNLIKELY	10 ⁻⁴	н	Μ	L	L	VL	
E - RARE	10 ⁻⁵	М	L	L	VL	VL	
F - BARELY CREDIBLE	10 ⁻⁶	L	VL	VL	VL	VL	

Notes: (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.

(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

RISK LEVEL IMPLICATIONS

	Risk Level	Example Implications (7)				
VH VERY HIGH RISK		Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.				
H HIGH RISK Unaccepta reduce risk		Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.				
м	MODERATE RISK	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.				
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.				
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.				

Note: (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide

Appendix C

Some Guidelines for Hillside Construction

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

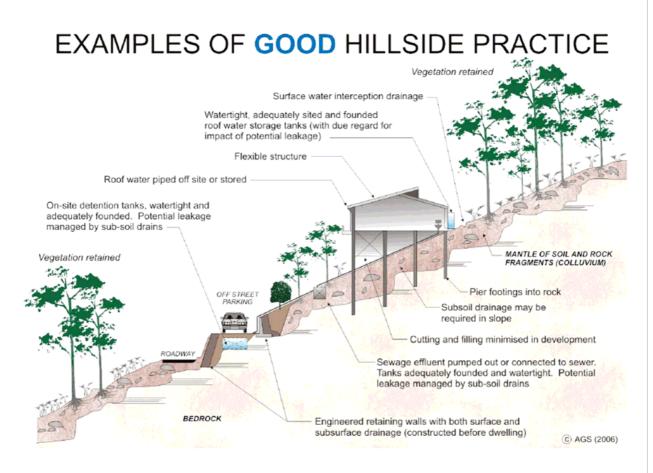
APPENDIX - SOME GUIDELINES FOR HILLSIDE CONSTRUCTION

GOOD ENGINEERING PRACTICE

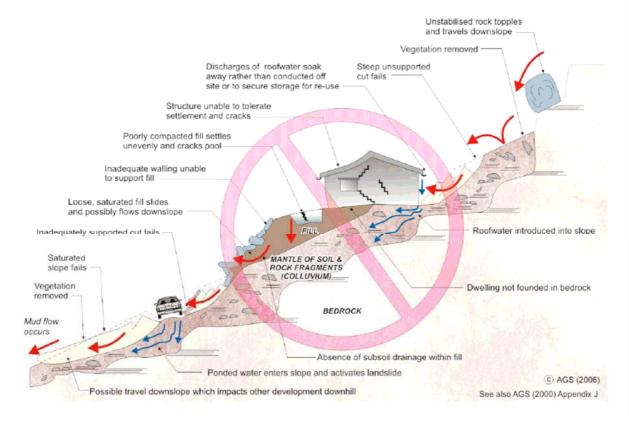
POOR ENGINEERING PRACTICE

	GOOD ENGINEERING PRACTICE	POOR ENGINEERING PRACTICE
ADVICE		
GEOTECHNICAL	Obtain advice from a qualified, experienced geotechnical practitioner at	Prepare detailed plan and start site works before geotechnical advice.
ASSESSMENT	early stage of planning and before site works.	geolechnical advice.
LANNING		
SITE PLANNING	Having obtained geotechnical advice, plan the development with the risk arising from the identified hazards and consequences in mind.	Plan development without regard for the Risk.
ESIGN AND CONSTR	RUCTION	
HOUSE DESIGN	Use flexible structures which incorporate properly designed brickwork, timber or steel frames, timber or panel cladding. Consider use of split levels. Use decks for recreational areas where appropriate.	Floor plans which require extensive cutting and filling. Movement intolerant structures.
SITE CLEARING	Retain natural vegetation wherever practicable.	Indiscriminately clear the site.
EARTHWORKS	Retain natural contours wherever possible.	Indiscriminatory bulk earthworks.
CUTS	Minimise depth. Support with engineered retaining walls or batter to appropriate slope. Provide drainage measures and erosion control.	Large scale cuts and benching. Unsupported cuts. Ignore drainage requirements
FILLS	Minimise height. Strip vegetation and topsoil and key into natural slopes prior to filling. Use clean fill materials and compact to engineering standards. Batter to appropriate slope or support with engineered retaining wall. Provide surface drainage and appropriate subsurface drainage.	Loose or poorly compacted fill, which if it fails, may flow a considerable distance including onto property below. Block natural drainage lines. Fill over existing vegetation and topsoil. Include stumps, trees, vegetation, topsoil, boulders, building rubble etc in fill.
ROCK OUTCROPS & BOULDERS	Remove or stabilise boulders which may have unacceptable risk. Support rock faces where necessary.	Disturb or undercut detached blocks or boulders.
RETAINING WALLS	Found on rock where practicable. Provide subsurface drainage within wall backfill and surface drainage on slope above. Construct wall as soon as possible after cut/fill operation.	Construct a structurally inadequate wall such as sandstone flagging, brick or unreinforced blockwork. Lack of subsurface drains and weepholes.
FOOTINGS	Found within rock where practicable. Use rows of piers or strip footings oriented up and down slope. Design for lateral creep pressures if necessary. Backfill footing excavations to exclude ingress of surface water.	Found on topsoil, loose fill, detached boulders or undercut cliffs.
SWIMMING POOLS	Engineer designed. Support on piers to rock where practicable. Provide with under-drainage and gravity drain outlet where practicable. Design for high soil pressures which may develop on uphill side whilst there may be little or no lateral support on downhill side.	
DRAINAGE		
SURFACE	Provide at tops of cut and fill slopes. Discharge to street drainage or natural water courses. Provide general falls to prevent blockage by siltation and incorporate silt traps. Line to minimise infiltration and make flexible where possible. Special structures to dissipate energy at changes of slope and/or direction.	Discharge at top of fills and cuts. Allow water to pond on bench areas.
SUBSURFACE	Provide filter around subsurface drain. Provide drain behind retaining walls. Use flexible pipelines with access for maintenance. Prevent inflow of surface water.	Discharge roof runoff into absorption trenches.
SEPTIC & SULLAGE	Usually requires pump-out or mains sewer systems; absorption trenches may be possible in some areas if risk is acceptable. Storage tanks should be water-tight and adequately founded.	Discharge sullage directly onto and into slopes. Use absorption trenches without consideration of landslide risk.
EROSION CONTROL & LANDSCAPING	Control erosion as this may lead to instability. Revegetate cleared area.	Failure to observe earthworks and drainage recommendations when landscaping.
	VISITS DURING CONSTRUCTION	1
DRAWINGS	Building Application drawings should be viewed by geotechnical consultant	
SITE VISITS	Site Visits by consultant may be appropriate during construction/	
ISPECTION AND MA	INTENANCE BY OWNER	
OWNER'S	Clean drainage systems; repair broken joints in drains and leaks in supply pipes.	
RESPONSIBILITY	Where structural distress is evident see advice. If seepage observed, determine causes or seek advice on consequences.	

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007



EXAMPLES OF **POOR** HILLSIDE PRACTICE



Australian Geomechanics Vol 42 No 1 March 2007

Appendix D

Certificate Forms



Engineering	Certificate					
To:	Mr Mark & Mrs Kim Phillips-Ha	aines		Owner /Agent		
	1 Walker Street			Address		
	Ulverstone Tas		715	Suburb/postcod	9	
Certifier details	:					
From:	Geoton Pty Ltd					
Address:	PO Box 522			Phone No:	(03) 6	326 5001
	Prospect		7250	Fax No:		
Accreditation No: (if applicable)	CC6220P	Er	nail address:	tbarriera@g	eoton.c	com.au
Or qualifications and Insurance details:	Tony Barriera – BEng, MSc, C IEAust 471929, Civil, Geotech Underwriters at Lloyd's- ENG	nical	Certain	(description from of Building Contr		
Speciality area of expertise:	Geotechnical Engineering Landslide Risk Assessments			(description from of Building Contr		
Details of work	:					
Address:	Harveys Road				Lot No:	2
	North Motton Tas		7315	Certificate of	f title No:	131560/1
The work related to this certificate:	Landslide Risk Assessment			(description of th certified)	ne work or	part work being
Certificate deta	ils:					
Certificate type:	Geotechnical			(description from of Building Conti		
In issuing this certifica	te the following matters are releva	nt –		1		
Documents:	Geoton Pty Ltd, Report Refere dated 28/08/2020.		No. GL203	322Ab,		
Relevant calculations:	Refer to report					
References:	Australian Geomechanics So Risk Management, 2007	ciety	- Practice	e Note Guide	elines fo	or Landslide



Substance of Certificate:

Findings and recommendations of report (Report Reference No. GL20322Ab).

From the Interim Planning Scheme 2013 the site is partially mapped within a Medium landslide hazard band. As such, a landslide risk assessment is required to determine if the level of risk from exposure to the landslide hazard is to be tolerable for the type, form, scale and duration of the development.

The landslide risk assessment was conducted in accordance with Australian Geomechanics Society (AGS) – Practice Note Guidelines for Landslide Risk Management, 2007. Our report concluded that the qualitative landslide risk for the site is at worst a LOW risk provided the development of the site is in accordance with the recommendations within our report. In our experience, regulating authorities allow developments to proceed with VERY LOW to LOW risk.

Therefore, provided the development of the site is in accordance with the recommendations within our report, then we consider that a tolerable level of risk can be achieved for the development of the site in accordance with section E6.6.2 (Development on land exposed to a natural hazard) of the Hazard Management Code of the Interim Planning Scheme 2013. That is, the level of likely risk from exposure to the natural hazard (landslide) is considered to be tolerable for the proposed residential development.

Scope or Limitations

The report provides a qualitative landslide risk assessment which identifies the landslide risks at the site and provides recommendations to maintain, improve and possibly reduce the risk of landslides so as not cause or contribute to the risk of landslides on the site and lands in the locality.

The site is within an area of inherent doubtful slope stability and landslides are a natural ongoing geological process. There will be always some level of landslide risk within an area of inherent doubtful slope stability. The recommendations of the report are provided to maintain, improve and possibly reduce the risk of landslides on the site and lands in the locality.

The recommendations for the design of the proposed works are in accordance with prevailing geological conditions described in the report for the site, assessed landslide risks and recommended good hillside practices.

I certify the matters described in this certificate.

Certifier:

	Signed:
1	
bhom	

28/08/2020

Date:

Certificate No.

GL20322Ab

CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

To:	Mr Mark & Mrs Kim Phillips-Haines	Owner /Agent	55	
	1 Walker Street	Address	Form 55	
	Ulverstone Tas 7	315	Suburb/postcode	9
Qualified perso	on details:			
Qualified person:	Tony Barriera - Geoton Pty. Ltd.]	
Address:	PO Box 522		Phone No:	03 6326 5001
	Prospect Tas 7	250	Fax No:	
Licence No:	CC6220 P Email addres	s: tba	rriera@geoto	n.com.au
Qualifications and Insurance details:	Tony Barriera – BEng, MSc CPEng, NER – IEAust 471929 Civil, Geotechnical Certain Underwriters at Lloyd's- ENG 19 000330	Deterr		a 3 of the Director's les by Qualified Persons
Speciality area of expertise:	Geotechnical Engineering		n 4 of the Director's tes by Qualified Persons	
Details of work				
Address:	Harveys Road]	Lot No: 2
	North Motton Tas 7	315	Certificate of	title No: 131560/2
The assessable item related to this certificate:	Classification of foundation conditions according to AS2870 - 2011		certified) Assessable item - a material; - a design - a form of co - a document - testing of a system or p	nstruction
Certificate deta	ils:			
Certificate type:	Foundation Site Classification – AS2870	Directo		1 of Schedule 1 of the Certificates by Qualified ems n)

This certificate is in relation to the above assessable item, at any stage, as part of - (tick one)

building work, plumbing work or plumbing installation or demolition work:

or

a building, temporary structure or plumbing installation:

Х

In issuing this certificate the following matters are relevant -

Documents:	Geoton Pty Ltd, Report Reference No. GL20322Ab, dated 28/08/2020
Relevant	Refer to report
calculations:	
References:	AS 2870 – 2011 Residential Slabs and Footings Construction
	AS 4055 – 2012 Wind Loads for Housing
	CSIRO Building Technical File 18
	Substance of Certificate: (what it is that is being certified)
	on in accordance to AS2870 - 2011
-	accordance to AS 4055 - 2012
Findings and rec	commendations of report

Scope and/or Limitations

The classification applies to the site as investigated at the time and does not account for any future alteration to foundation conditions resulting from earthworks, drainage condition changes or site maintenance variations.

I certify the matters described in this certificate.

	Signed:	 Certificate No:	Date:
Qualified person:	bann	GL20322Ab	28/08/2020



LOADING CERTIFICATE

To:	Mr Mark & Mrs Kim Phillips-	Mr Mark & Mrs Kim Phillips-Haines		Certificate Ref: AS/NZS 1547:2012		
	1 Walker Street		Address	Section 7.4.2		
	Ulverstone Tas	7315	Suburb/postcode			

Details of work:

Address:	Harveys Road		Lot No: 2
	North Motton Tas 73	315	Certificate of title No: 131560/2
The work related to this certificate:	On-site domestic-wastewater management		(description of the work or part work being certified)

Certificate details:

In issuing this certificate the following matters are relevant -

Documents:	Report GL20322Ab dated 28/08/2020 Figure 1 - Site Plan Figure WW-02 - Absorption Trench Section
Relevant calculations:	Contained in the above
References:	AS/NZS1547:2012 On-site domestic-wastewater management

Substance of Certificate:

This certificate sets out the design criteria and the limitations associated with use of the system.

Wastewater Characteristics

Population equivalent used for this assessment	= 5 (3 Bedrooms)
Wastewater volume (L/day) used for this assess	sment = 600 (120 Litres per person)
Approximate blackwater volume (L/day)	= 250
Approximate greywater volume (L/day)	= 350

Soil Characteristics/Design Criteria

Texture (Table E4 from AS/NZS 1547)	= Clay Loam
Soil category (Table E1 from AS/NZS 1547)	= 4
Soil structure (Table E4 from AS/NZS 1547)	= Moderately Structured
Indicative permeability (Table 5.1 from AS/NZS 1547	7) = 0.5m/day - 1.5m/day
Adopted permeability	= 0.6m/day
Adopted Design Loading Rate	= 15mm/day
Soil thickness for disposal	= >1.5m
Minimum depth (m) to water	= >1.5m
	= >1.5m

Dimensions for On-Site Treatment System

Disposal and treatment methods	= Septic tank minimum capacity 3000L
Site modification and specific design	P = N/A
Trench Length	= 40m (2 x 20m trenches)
Trench Width	= 1.0m
Trench Depth	= 0.45m
Primary disposal area required	= 216m ²
Reserve disposal area required	= 216m ²
Location and use of Reserve area	= Reserve area located to the north of the
proposed wastewater disposal area.	
Is there sufficient area available on s	site for disposal (including reserve) = Yes
	,

Notes

The purpose of the reserve area is to allow for future extension of the land application system to allow a factor of safety against unforseen malfunction or failure, perhaps following increased household occupancy or inadvertent misuse of the system.

The land application area may be reduced to account for flow reductions by water-saving devices, provided the organic loading rate is not higher than it would have been without the flow reduction.

Allowable Variation from Design Flow

Based on a septic tank capacity of 3000L and wastewater design volume of 600L/day the allowable variation from design flow (peak loading events) would be an additional 400L/day (Total flow of 1000L/day as per table J1 of AS/NZS 1547:2012).

System Limitations

Consequences of overloading the system:

Overloading the system can result in failure of the septic tank and land application system. This is a serious health and environmental hazard and can lead to any one or more of the following: Spread of infectious disease; Breeding of mosquitoes and attraction of flies and rodents; Nuisance and unpleasantness; Pollution of waterways; Contamination of bores, wells and groundwater; and alteration to local ecology.

Consequences of under loading the system:

Under loading the system may result in the bacteria to stop working and system failure.

Operation Requirements

Refer to Section T5.2.1 of AS/NZS 1547:2012 for additional requirements. For on-site system to work well the following is required:

- Reduce sludge building up through scraping all dishes to remove fats/grease; don't use a food waste disposal unit; and don't put sanitary napkins into the system.
- To keep bacteria working in the septic tank use biodegradable soaps; use a low phosphorous detergent; don't use powerful bleaches and disinfectants; and don't put chemicals or paint down the drain.
- Conservation of water will reduce the volume of effluent requiring disposal to the land application area, make it last longer and improve its performance.

Maintenance Requirements

Refer to Section T5.2.2 of AS/NZS 1547:2012 for additional requirements. Maintenance of the system should include the following:

Septic tanks must be inspected at least annually and pumped out regularly once

the scum and sludge occupy two thirds of the tank volume. Typically a septic tank must be pumped out at least every 3 to 5 years or more frequently depending on usage.

- Grease traps must be inspected at least quarterly and cleaned out regularly.
- Deep rooting trees or shrubs should not be grown over absorption trenches or pipes.
- Surface water diversion drains should be maintained upslope of and around the land application area and kept clean to reduce seepage of rainwater into the trenches.
- Maintain disposal area by maintaining plants and mowing grass to ensure that plants/grasses take up nutrients with maximum efficiency.
- Check disposal area for blockages such as wet spots and uneven grass colour.

I certify the matters described in this certificate.

Certifier:

Signed: M.M.M

28/08/2020 G

Date:

GL20322Ab

Certificate No.

CERTIFICAT	E OF THE RESPONS	SIBLE DESIG	NER	Section 106 Section 129 Section 155
То:	Mr Mark & Mrs Kim Phillips-Haines		Owner name	• -
10.	1 Walker Street		Address	Form 35
	Ulverstone Tas	731	5 Suburb/postcode	
[731	<u> </u>	
Designer detail	s:			
Name:	Matthew Street		Category:	Civil Engineer Hydraulic - Domestic
Business name:	Geoton Pty Ltd		Phone No:	03 6326 5001
Business address:	P O Box 522			
	Prospect TAS	725	0 Fax No:	
Licence No:	CC6221N Email a	address: mstreet	@geoton.com.a	U
Details of the p	roposed work:			
	Γ		Designer's project	
Owner/Applicant	Mr Mark & Mrs Kim Phi	illips-Haines	reference No.	^{ct} GL20322Ab
Address:	Harveys Road		Lot No:	2
	North Motton Tas	731	5	
Type of work:	Building w	ork	Plumbing work	X (X all applicable)
Description of wo New building on-site wastewate	er management system		add re- wa sto on- ma	ew building / alteration / dition / repair / removal / erection ater / sewerage / mwater / -site wastewater anagement system / oldewaster (stear)
Description of the	Design Work (Scope, limita	ations or exclusion		ckflow prevention / other) certificates)
Certificate Type:	Certificate		Responsible Prac	ctitioner
	Building design		Architect or Buildin	ng Designer
	Structural design		Engineer or Civil D	Designer
	☐ Fire Safety design		Fire Engineer	
	Civil design		Civil Engineer or C	
	 Hydraulic design Fire service design 		Building Services I Building Services I	
	Electrical design		Building Services I	
	Mechanical design		Building Service D	
	□ Plumbing design		Plumber-Certifier;	Architect, Building
			Designer or Engine	eer
	Other (specify)			
Deemed-to-Satisfy:	×	Performance S	olution: D (X the	e appropriate box)
Other details: All design docu	uments provided in Rep	oort GL20322A	b, dated 28/08/	2020
Design docum	ents provided:			

Director of Building Control - date approved: 2 August 2017

Section 94

The following documents are provided with this Certificate -

Document description:		
Drawing numbers:	Prepared by:	Date:
Schedules:	Prepared by:	Date:
Specifications:	Prepared by:	Date:
Computations:	Prepared by:	Date:
Performance solution proposals:	Prepared by:	Date:
Test reports:	Prepared by:	Date:

Standards, codes or guidelines relied on in design	
process:	
All design documents are contained within report	
AS/NZS1547:2012 On-site domestic-wastewater management	

Any other relevant documentation:	

Attribution as designer:

I Matthew Street of Geoton Pty Ltd am responsible for the design of that part of the work as described in this certificate;

The documentation relating to the design includes sufficient information for the assessment of the work in accordance with the *Building Act 2016* and sufficient detail for the builder or plumber to carry out the work in accordance with the documents and the Act;

This certificate confirms compliance and is evidence of suitability of this design with the requirements of the National Construction Code.

	Name: (print)	Signed	Date
Designer:	Matthew Street	M.M.M	28/08/2020
Licence No:	CC6221N		

Assessment of Certifiable Works: (TasWater)
Note: single residential dwellings and outbuildings on a lot with an existing sewer connection are not considered to increase demand and are not certifiable.
If you cannot check ALL of these boxes, LEAVE THIS SECTION BLANK.
TasWater must then be contacted to determine if the proposed works are Certifiable Works.
I confirm that the proposed works are not Certifiable Works, in accordance with the Guidelines for TasWater CCW Assessments, by virtue that all of the following are satisfied:
The works will not increase the demand for water supplied by TasWater
The works will not increase or decrease the amount of sewage or toxins that is to be removed by, or discharged into, TasWater's sewerage infrastructure
The works will not require a new connection, or a modification to an existing connection, to be made to TasWater's infrastructure
The works will not damage or interfere with TasWater's works
The works will not adversely affect TasWater's operations
The work are not within 2m of TasWater's infrastructure and are outside any TasWater easement
I have checked the LISTMap to confirm the location of TasWater infrastructure
If the property is connected to TasWater's water system, a water meter is in place, or has been applied for to TasWater.

Certification:

I Matthew Street of Geoton Pty Ltd being responsible for the proposed work, am satisfied that the works described above are not Certifiable Works, as defined within the *Water and Sewerage Industry Act 2008,* that I have answered the above questions with all due diligence and have read and understood the Guidelines for TasWater CCW Assessments.

Note: the Guidelines for TasWater Certification of Certifiable Works Assessments are available at: <u>www.taswater.com.au</u>

Designer:

Matthew Street

Name: (print)



Date

28/08/2020